

I. Introduction

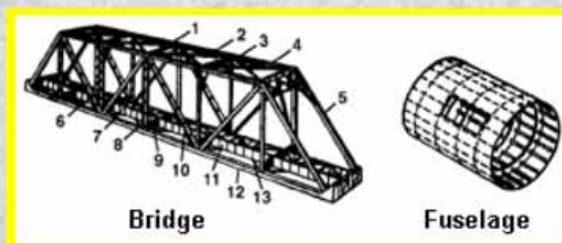
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- 1.2 Relations Between External Forces and Response Quantities
- 1.3 Classification of Structural Members (According to Spatial Extent)
- 1.4 Relationship Between Mechanics of Materials and Other Disciplines
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 - b) Mechanics Disciplines
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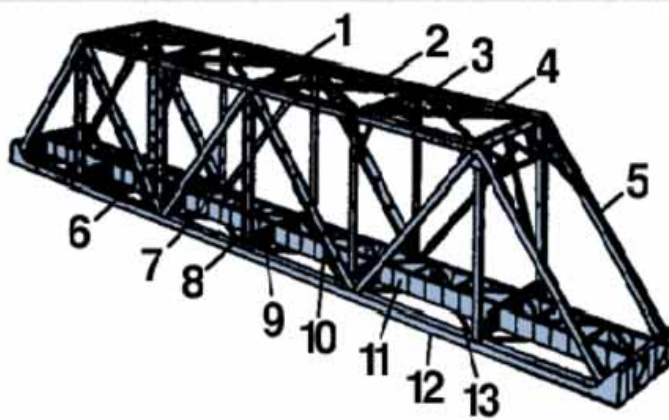
Definitions

Mechanics of Materials

Deals with

Art of **idealizing** actual solids and structures and their environments





Bridge



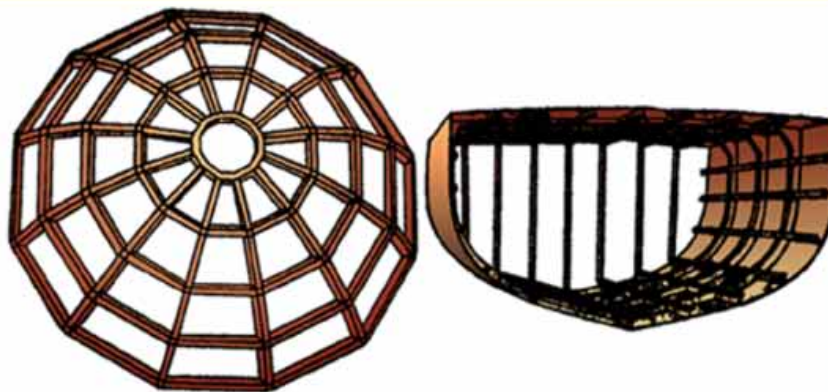
Fuselage

Definitions

Mechanics of Materials

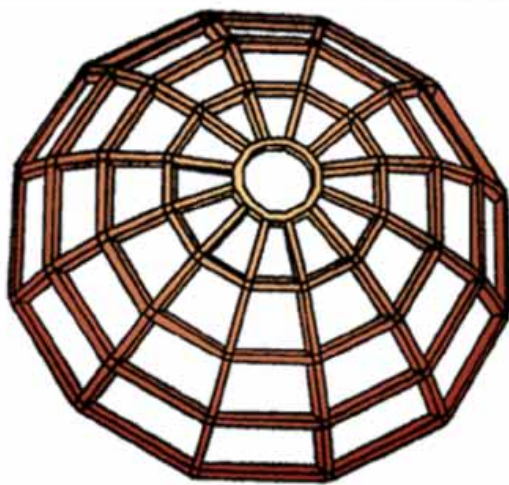
Deals with

Art of **idealizing** actual solids and structures and their environments



Roof shell

Ship's hull



Roof shell



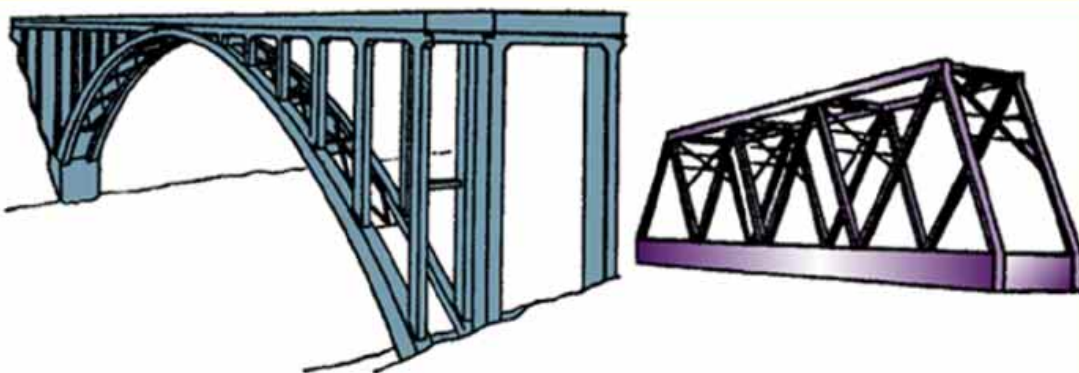
Ship's hull

Definitions

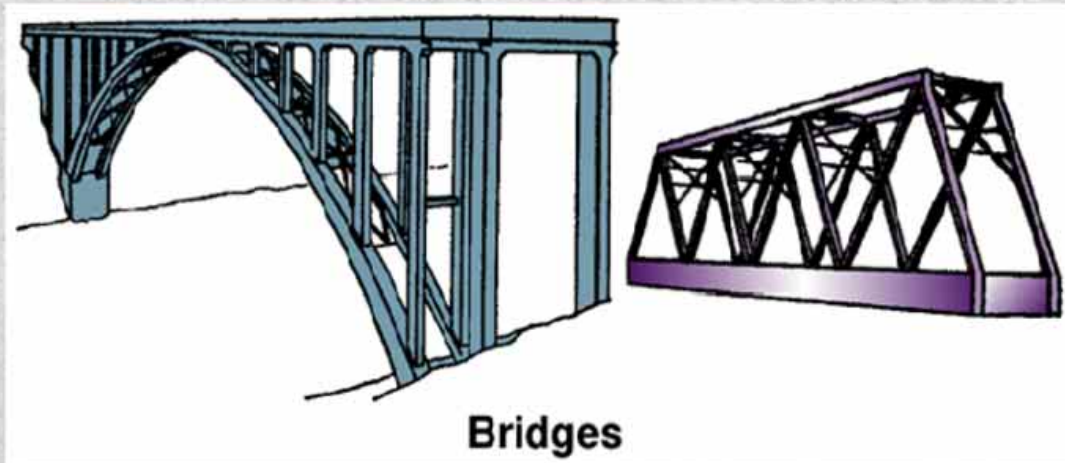
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Bridges



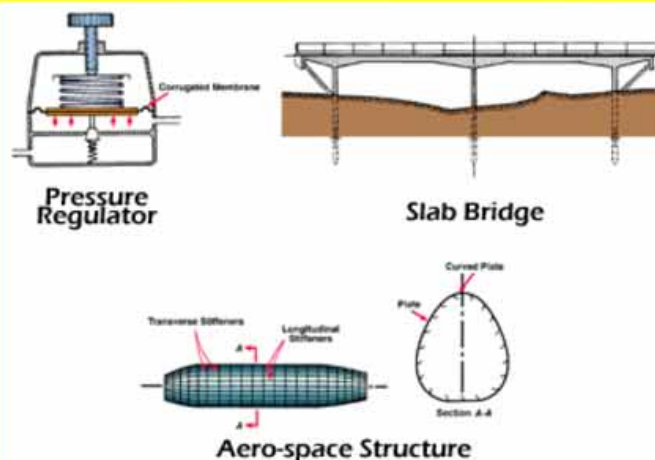
Bridges

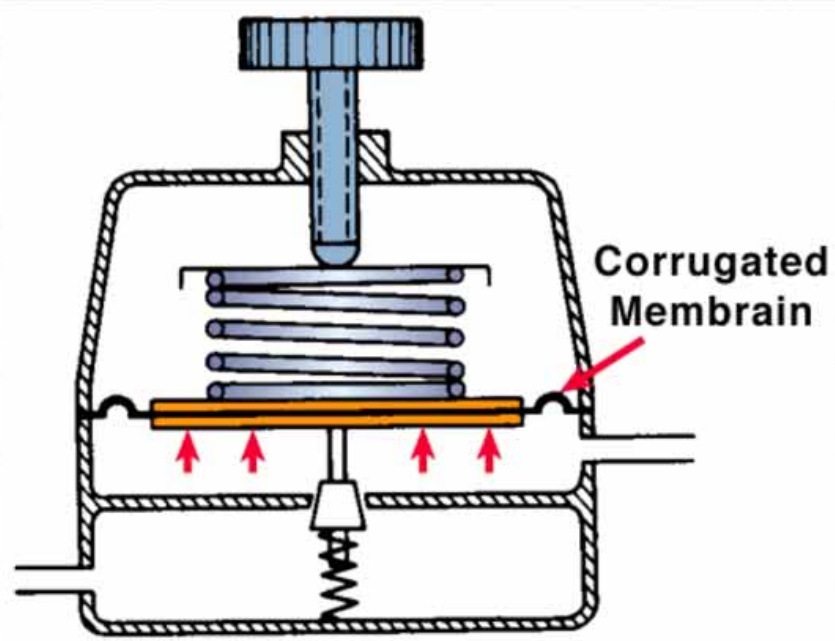
Definitions

Mechanics of Materials

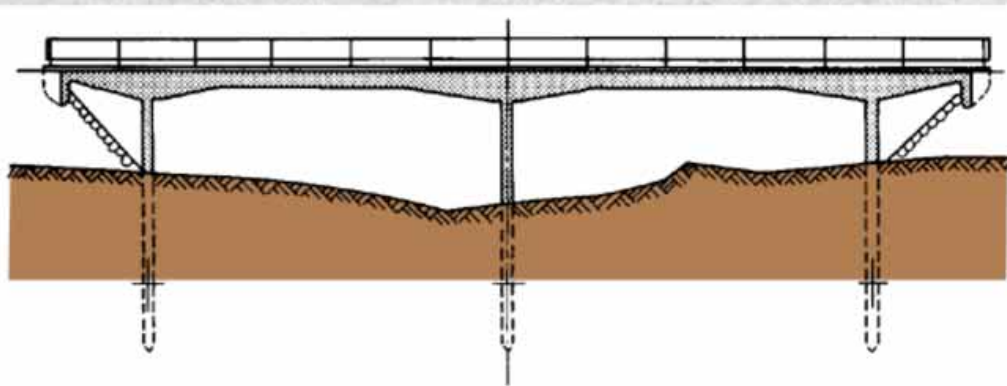
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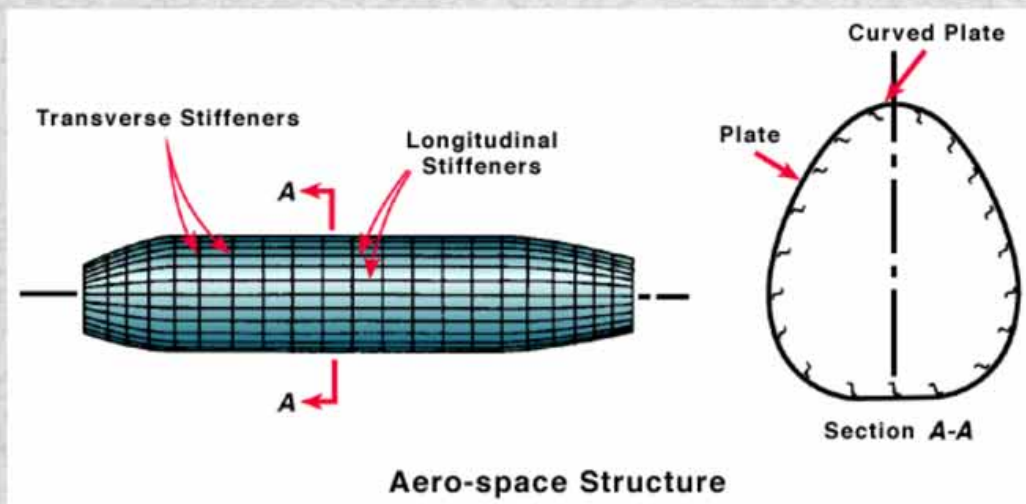




Pressure Regulator



Slab Bridge

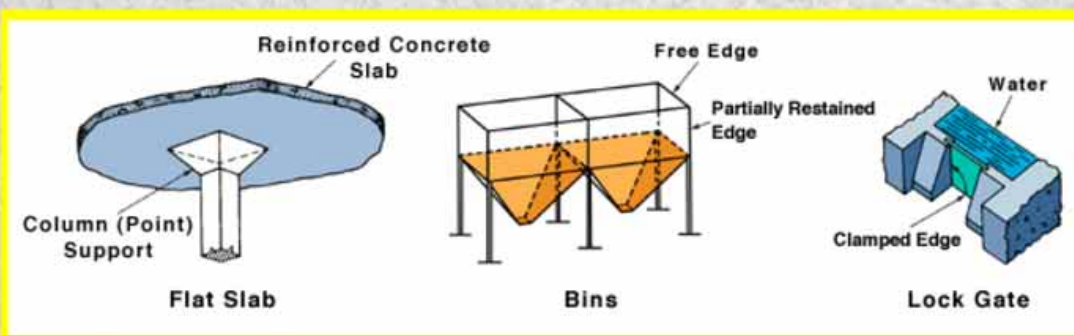


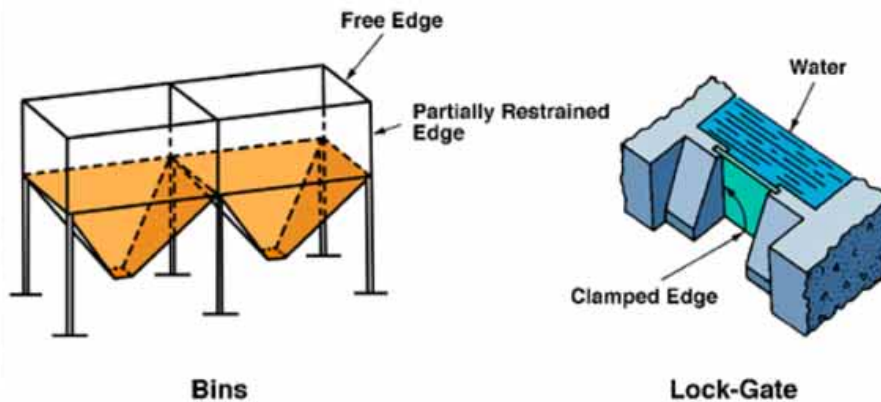
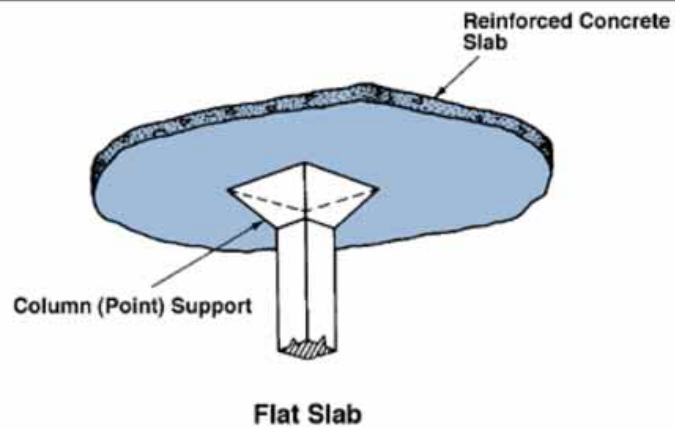
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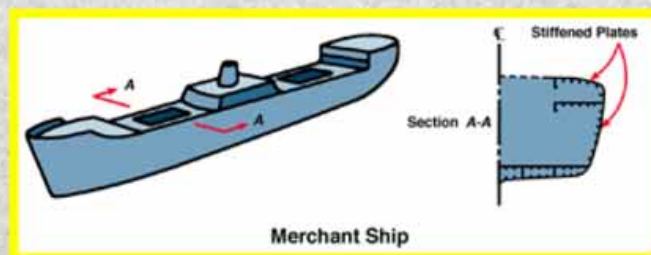


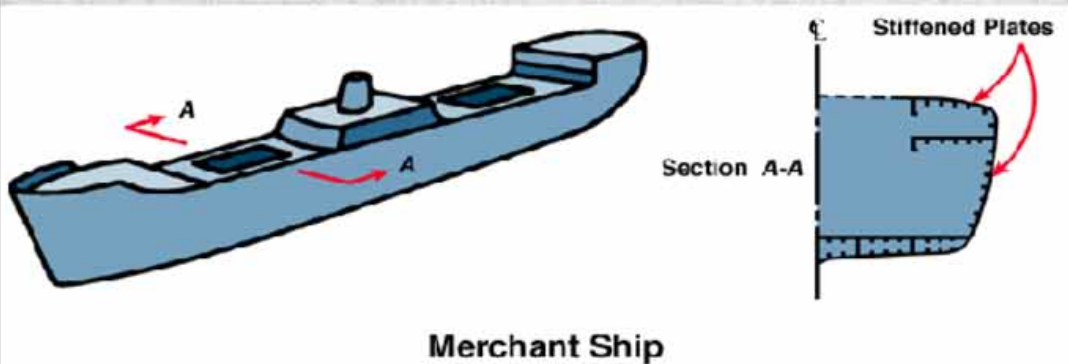
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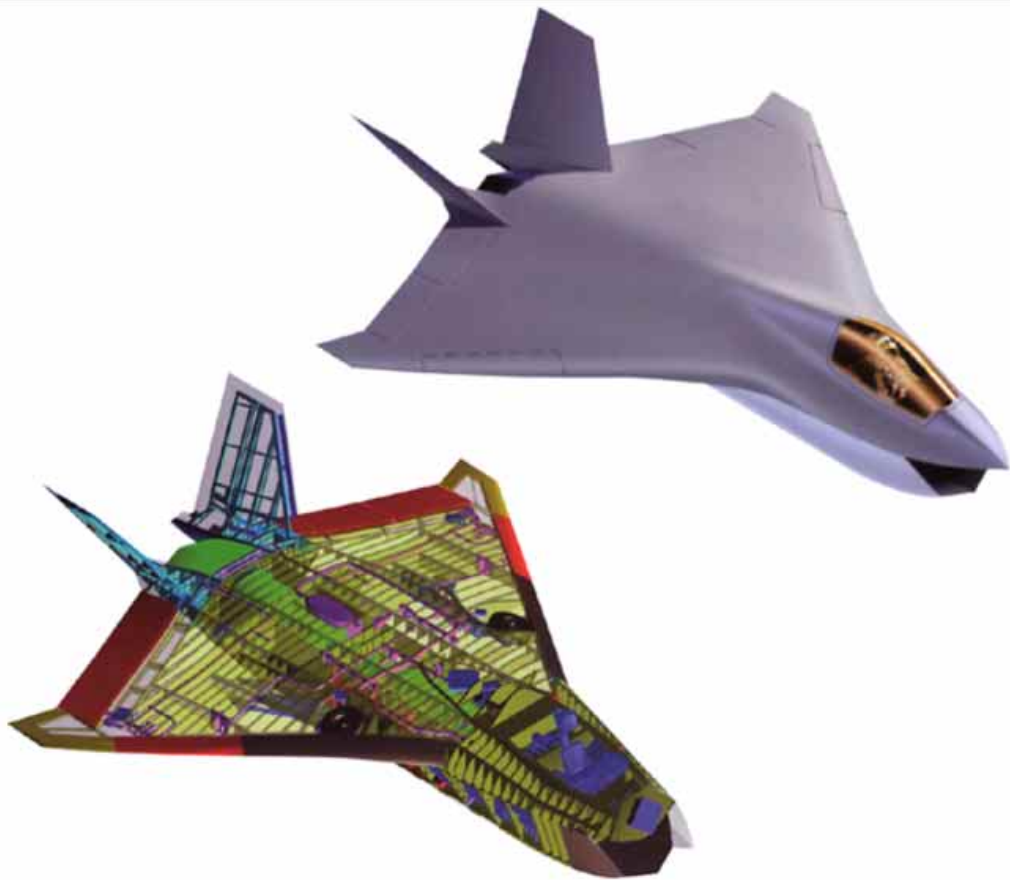
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Art of **idealizing** actual solids and structures and their environments





Definitions

Mechanics of Materials

Deals with

Art of **idealizing** actual solids and structures and their environments





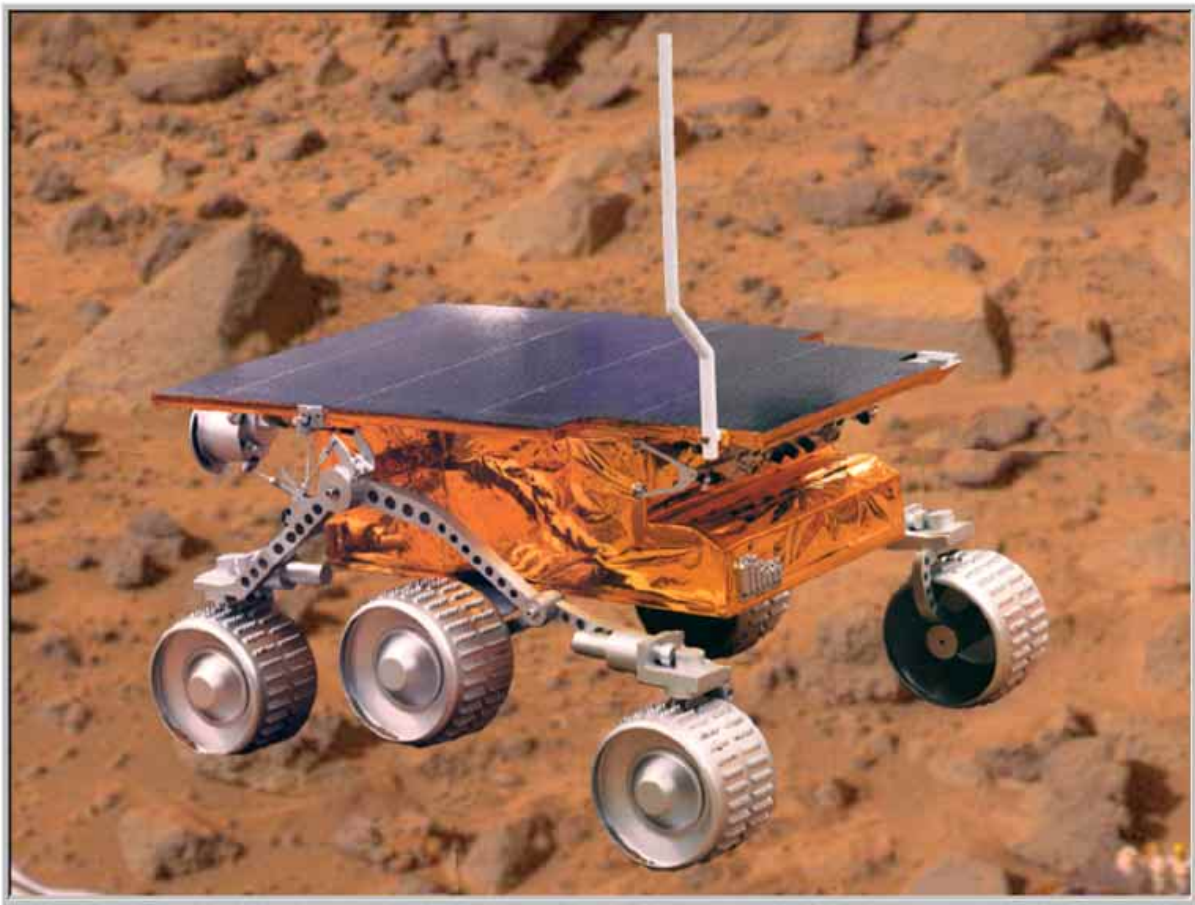
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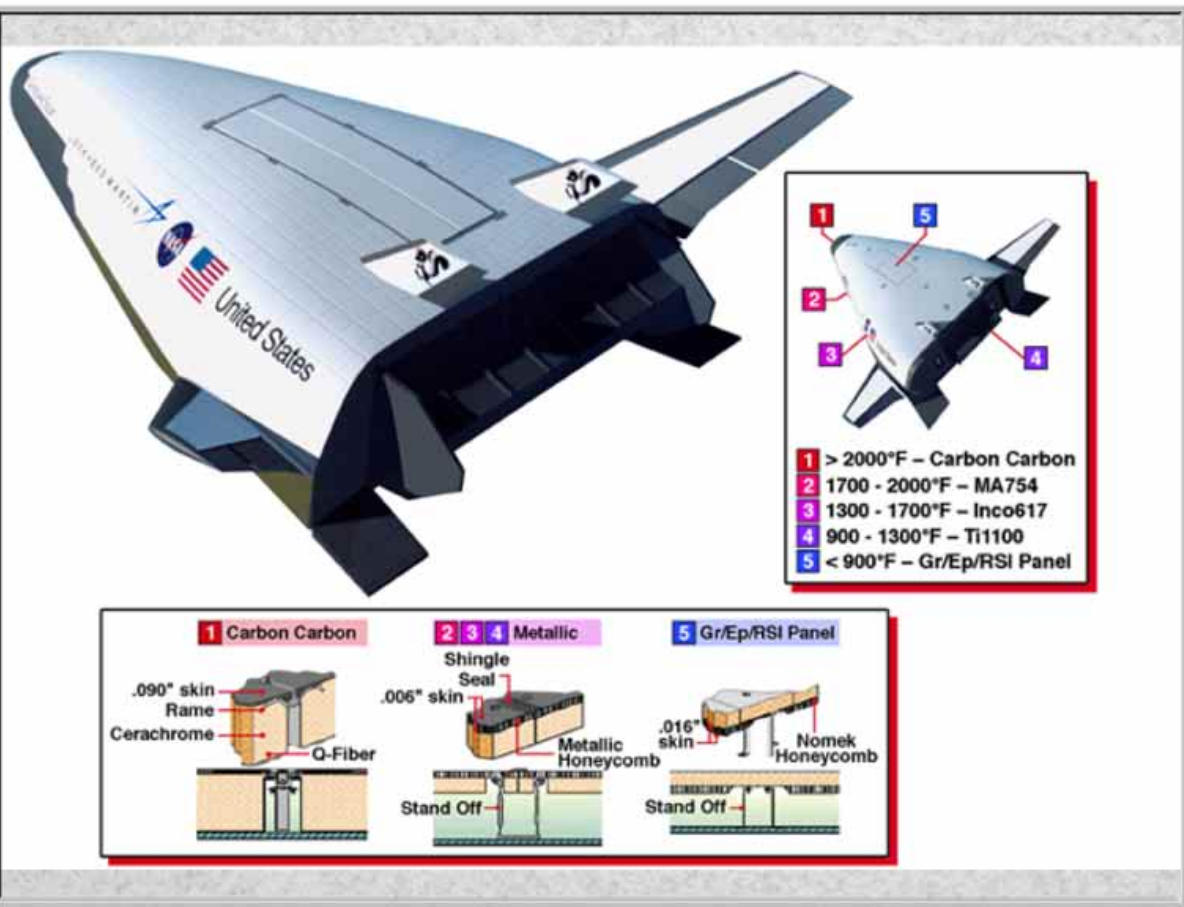
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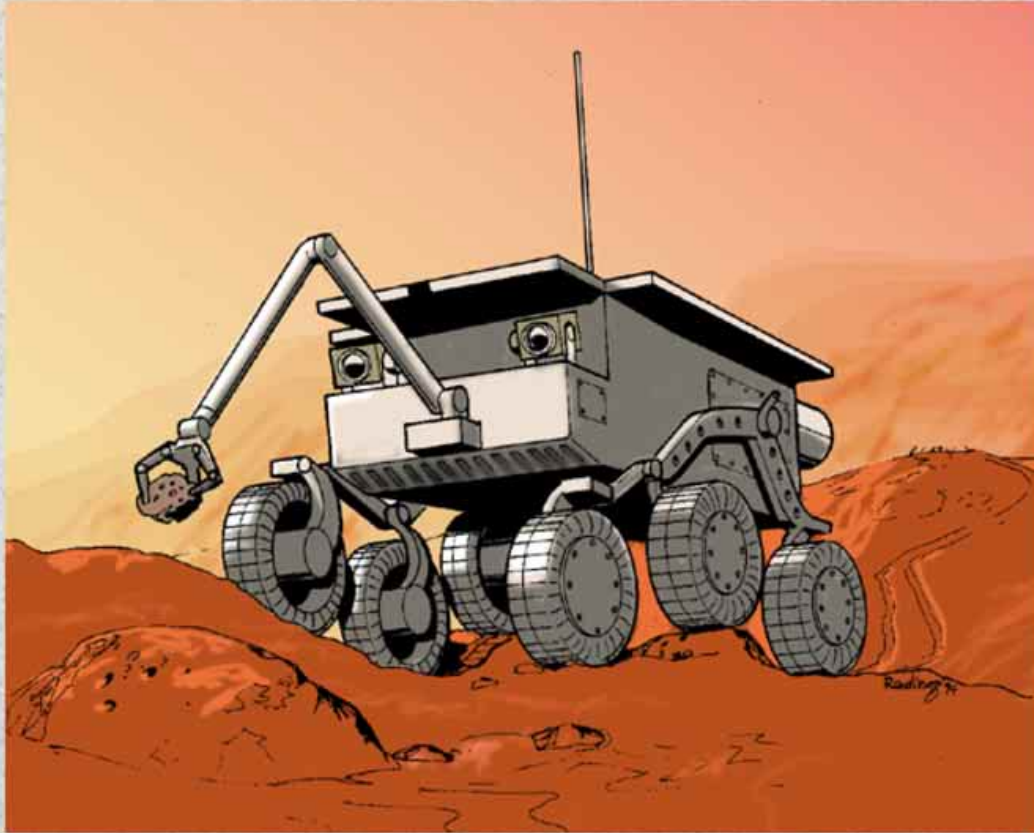
Definitions

Mechanics of Materials

Deals with

Art of **idealizing** actual solids and structures and their environments





Definitions

Mechanics of Materials

Deals with

Prediction of *response, life* and *failure* of structures and components thereof using **simplified theories.**

Definitions

Response

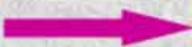
Measured in terms of
displacements, **v**elocities,
strains and **s**tresses.

Definitions

Response

Functions which govern
response can be grouped into:

Kinematic variables



Kinetic variables

Material characteristics

Source variables

displacements
velocities
strains
strain rates

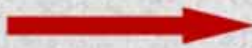
Definitions

Response

Functions which govern response can be grouped into:

Kinematic variables

Kinetic variables



**stresses
internal forces**

Material characteristics

Source variables

Definitions

Response

Functions which govern response can be grouped into:

Kinematic variables

Kinetic variables

Material characteristics



**stiffnesses
compliances
flexibilities**

Source variables

Definitions

Response

Functions which govern response can be grouped into:

Kinematic variables

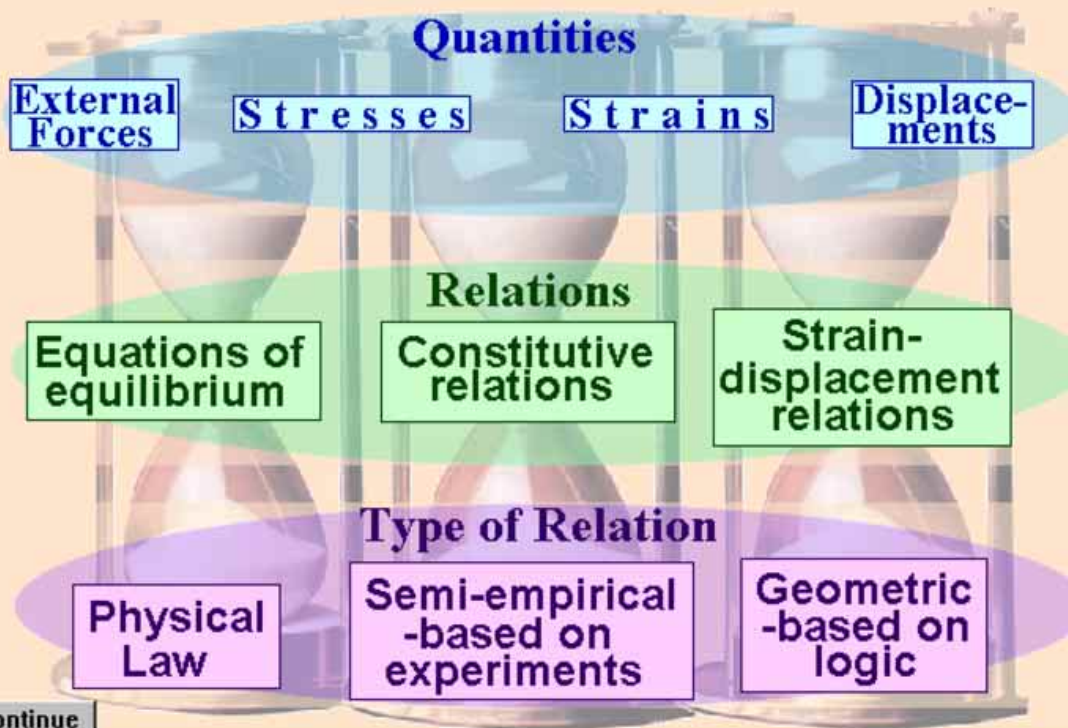
Kinetic variables

Material characteristics

Source variables →

mechanical, environmental forces (mechanical, aerodynamic, thermal, optical and electromagnetic forces)

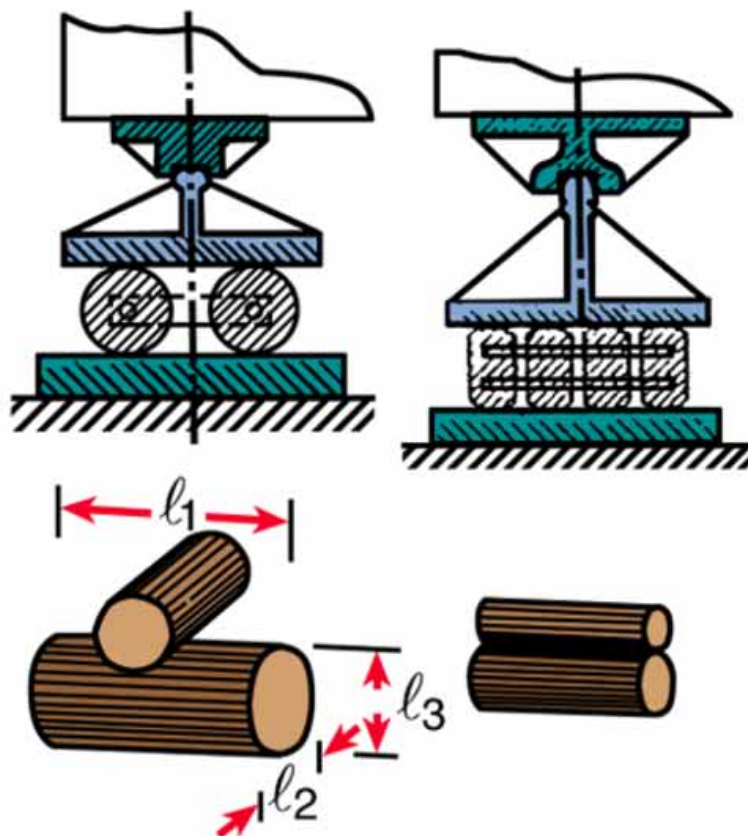
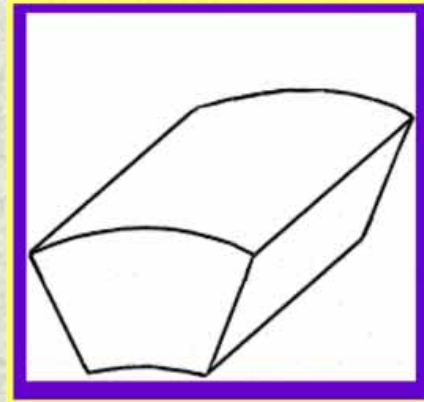
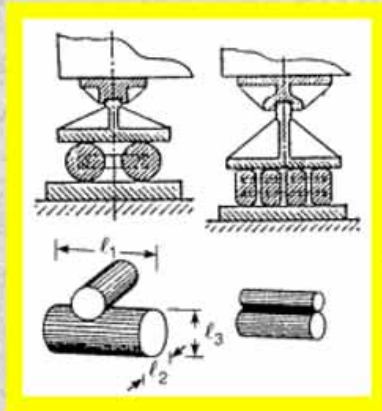
Relations Between External Forces and Response Quantities



Classification of Structural Members (according to Spatial Extent)

Three-dimensional members

$$\ell_1 = O(\ell_2) = O(\ell_3)$$



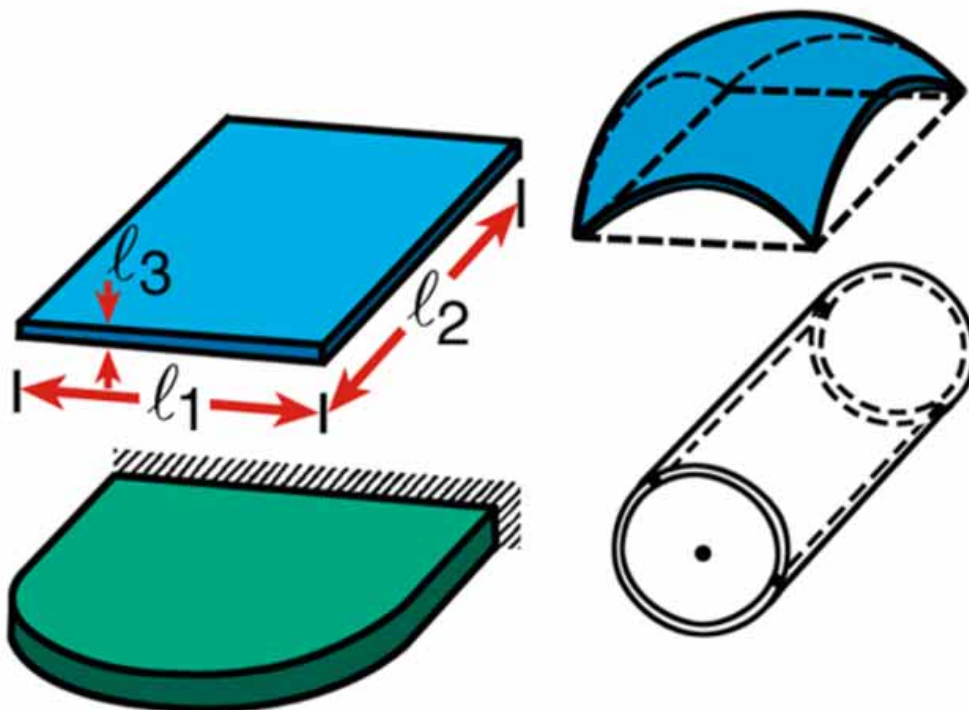
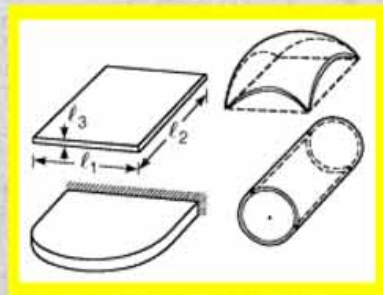
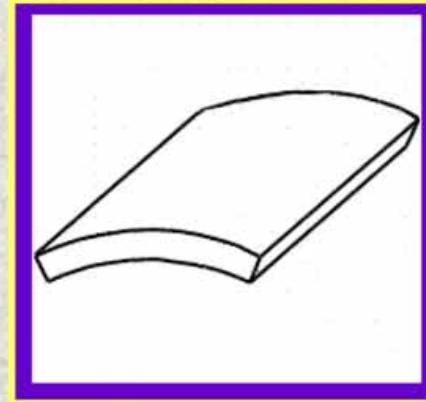
Classification of Structural Members (according to Spatial Extent)

Three-dimensional members

$$\ell_1 = O(\ell_2) = O(\ell_3)$$

Two-dimensional members

$$\ell_1 = O(\ell_2) \gg \ell_3$$



Classification of Structural Members (according to Spatial Extent)

Three-dimensional members

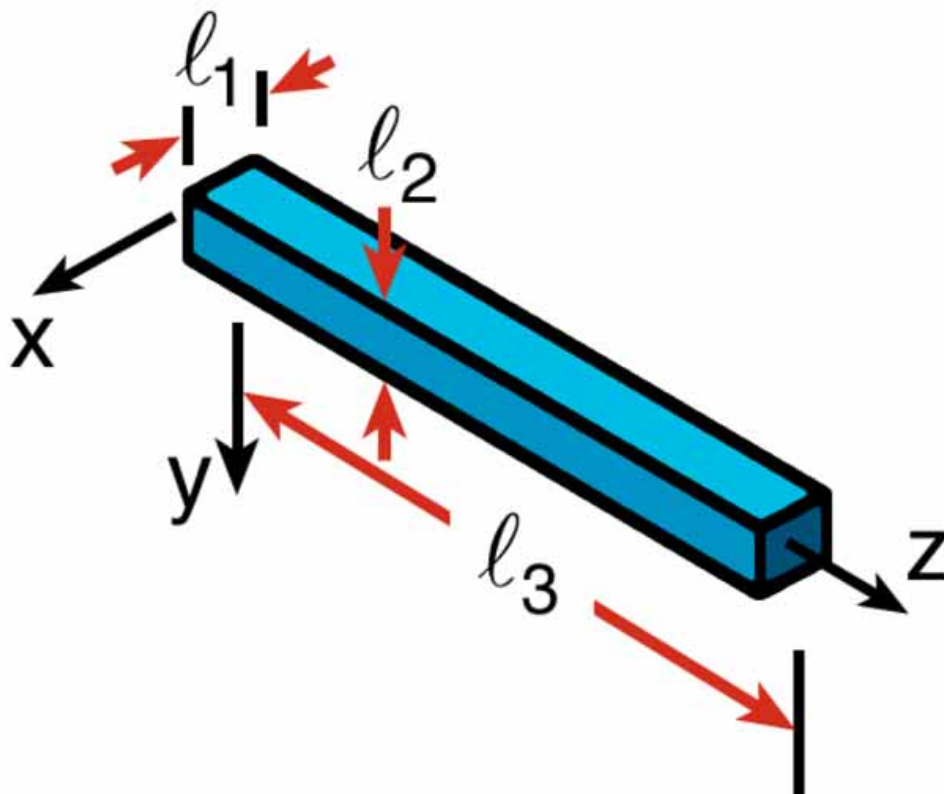
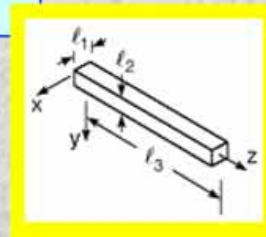
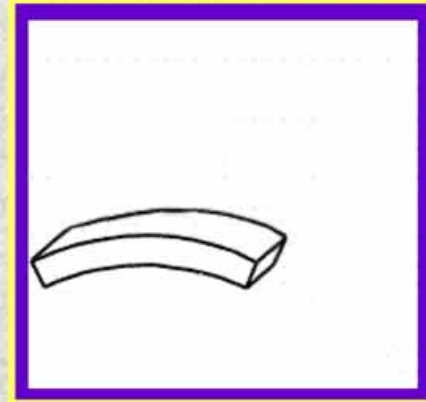
$$l_1 = O(l_2) = O(l_3)$$

Two-dimensional members

$$l_1 = O(l_2) \gg l_3$$

One-dimensional members

$$l_2 = O(l_1) \ll l_3$$



Classification of Structural Members (according to Spatial Extent)

Three-dimensional members

$$l_1 = O(l_2) = O(l_3)$$

Two-dimensional members

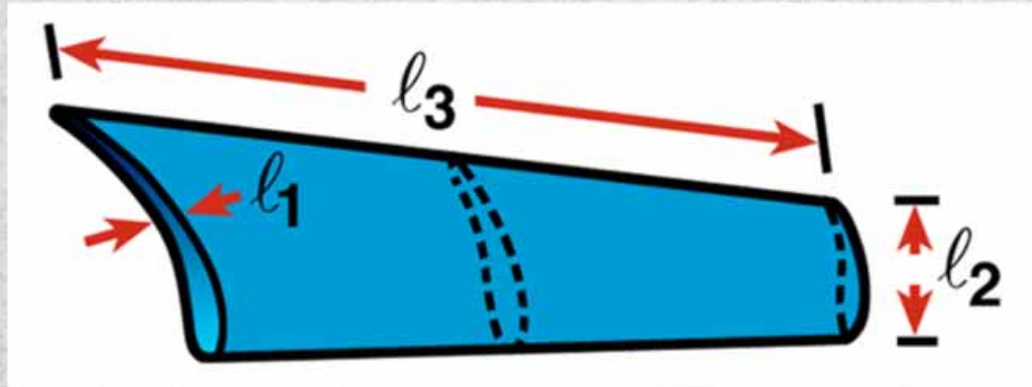
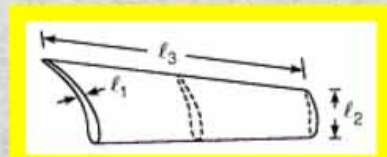
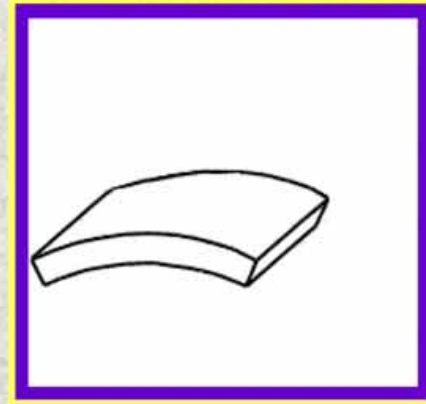
$$l_1 = O(l_2) \gg l_3$$

One-dimensional members

$$l_2 = O(l_1) \ll l_3$$

Thin-walled beams

$$l_3 \gg l_2 \gg l_1$$



Relationship Between Mechanics of Materials and Other Disciplines

LEVEL OF INVESTIGATION TYPE OF MATERIAL		ATOMIC AND MOLECULAR	MICROSCOPIC	MACROSCOPIC (PHENOMENOLOGICAL)	
CRYSTALLINE	NONMETALLIC	PHYSICAL CHEMISTRY	CRYSTALLOGRAPHY	ROCK MECHANICS	MATERIALS TESTING
	METALLIC	SOLID STATE PHYSICS	STRUCTURAL THEORIES OF DEFORMATION	<i>Mechanics of the Solid Continuum</i>	
		PHYSICAL METALLURGY	MECHANICAL METALLURGY	STRUCTURAL MECHANICS	
"AMORPHOUS"	GLASS AND POLYMER	POLYMER PHYSICS	STRUCTURAL RHEOLOGY	(ELASTICITY, PLASTICITY, RHEOLOGY)	
	POLYPHASE (HETEROGENEOUS)	COLLOID CHEMISTRY		MECHANICS OF GRANULAR MEDIA	
		SURFACE CHEMISTRY		SOIL MECHANICS	



Relationship Between Mechanics of Materials and Other Disciplines

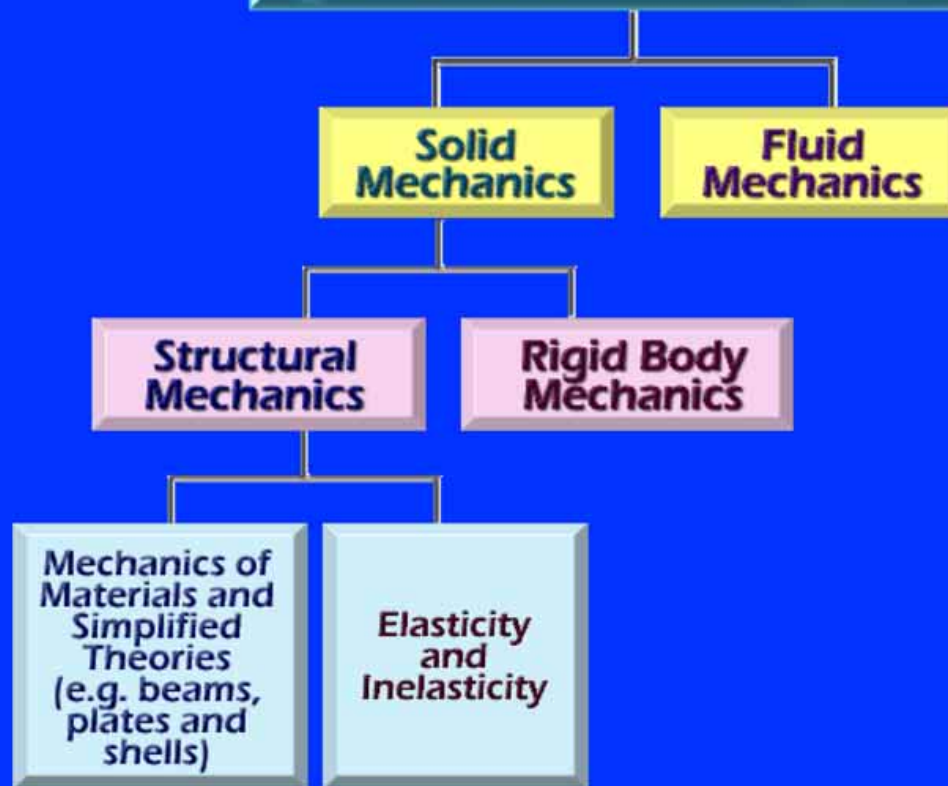
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		PHYSICAL METALLURGY	MECHANICAL METALLURGY	STRUCTURAL MECHANICS	
"AMORPHOUS"	GLASS AND POLYMER	POLYMER PHYSICS	STRUCTURAL RHEOLOGY	(ELASTICITY, PLASTICITY, RHEOLOGY)	
	POLYPHASE (HETEROGENEOUS)	COLLOID CHEMISTRY		MECHANICS OF GRANULAR MEDIA	
		SURFACE CHEMISTRY		SOIL MECHANICS	

**Mechanics
of the Solid
Continuum**

**Structural
Mechanics**
(Elasticity,
Plasticity,
Rheology)

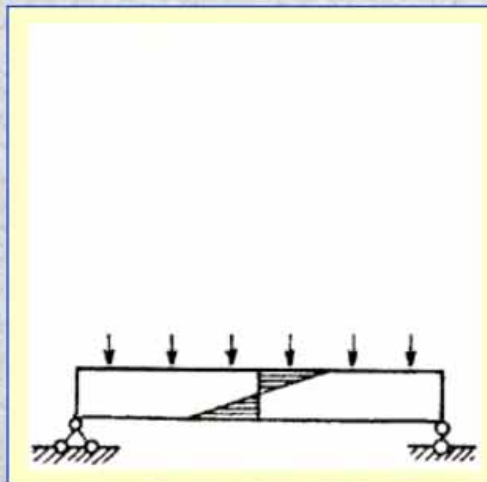


Continuum Mechanics



Mechanics of Materials vs. Elasticity and Inelasticity

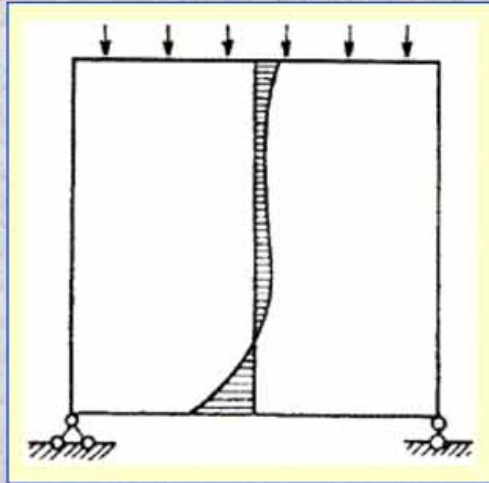
Examples of problems for which mechanics of materials assumptions are not valid



Shallow Beams

Mechanics of Materials vs. Elasticity and Inelasticity

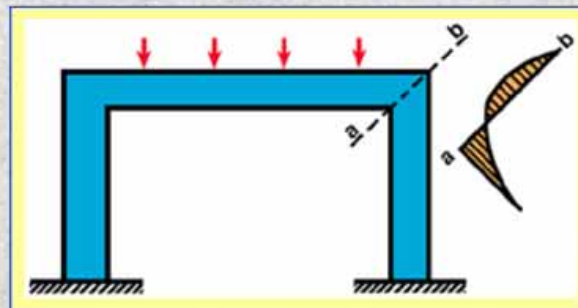
Examples of problems for which mechanics
of materials assumptions are not valid



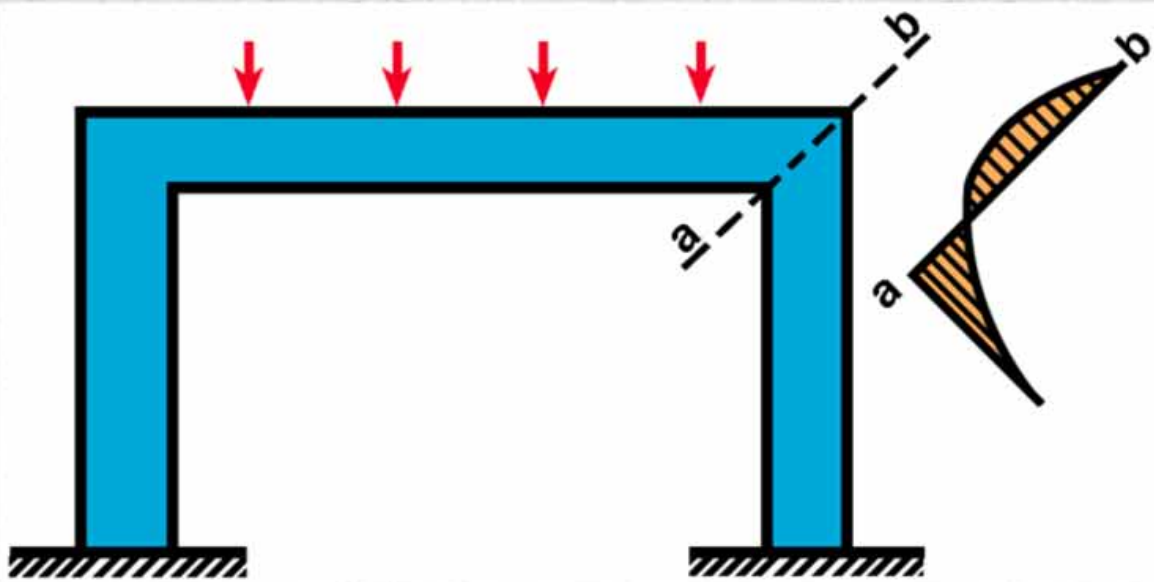
Deep Beams

Mechanics of Materials vs. Elasticity and Inelasticity

Examples of problems for which mechanics
of materials assumptions are not valid



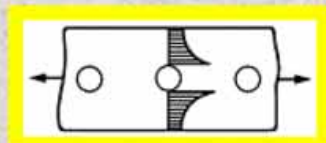
Stresses at frame joints



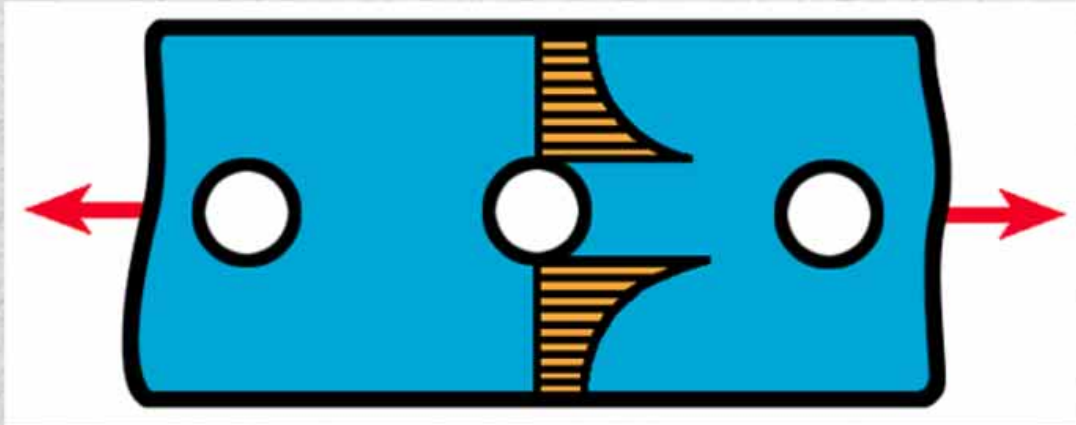
Mechanics of Materials vs. Elasticity and Inelasticity

Examples of problems for which mechanics of materials assumptions are not valid

Stress concentrations



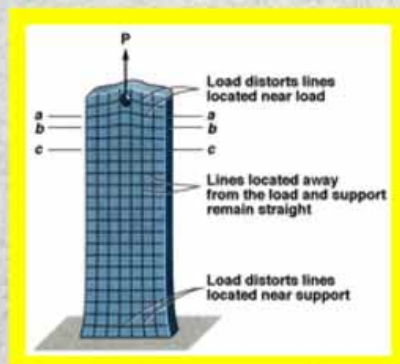
- Near discontinuities (cutouts and sharp changes)
- Near points of application of loads



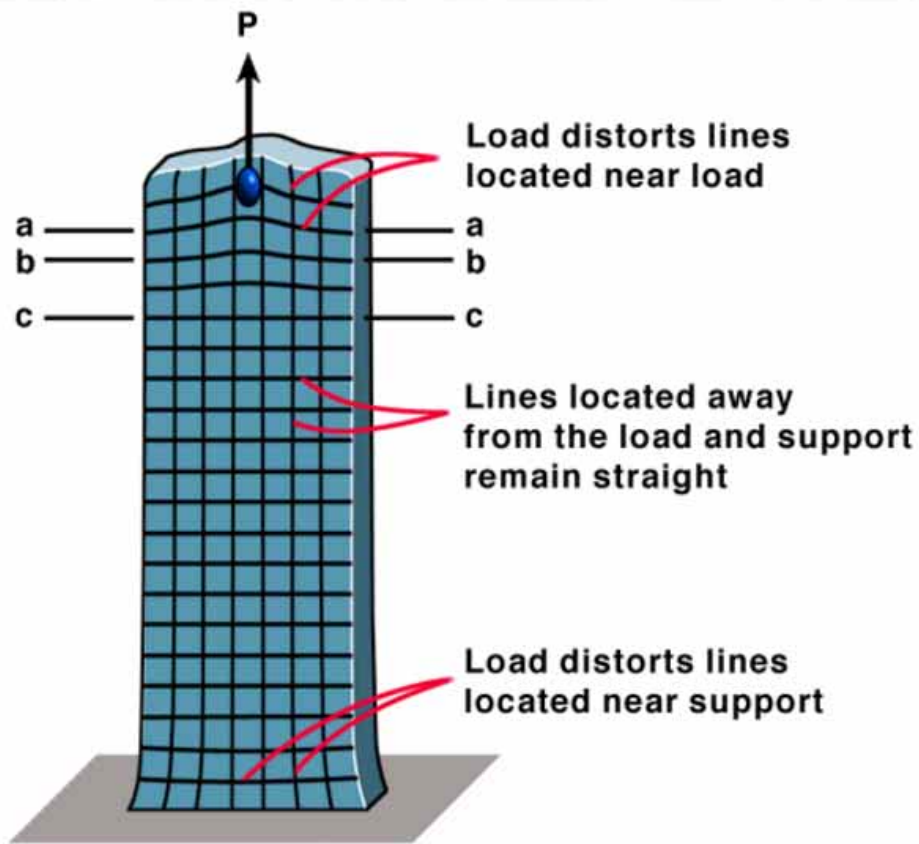
Mechanics of Materials vs. Elasticity and Inelasticity

Examples of problems for which mechanics of materials assumptions are not valid

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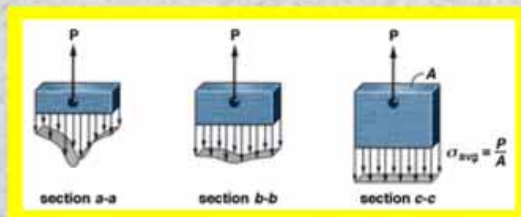
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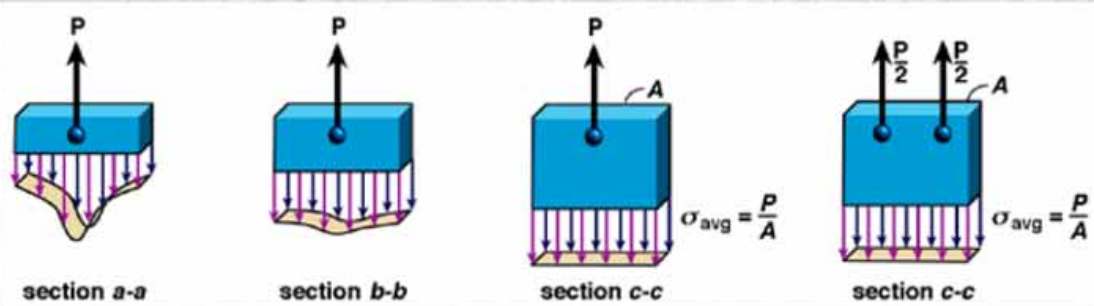
Mechanics of Materials vs. Elasticity and Inelasticity

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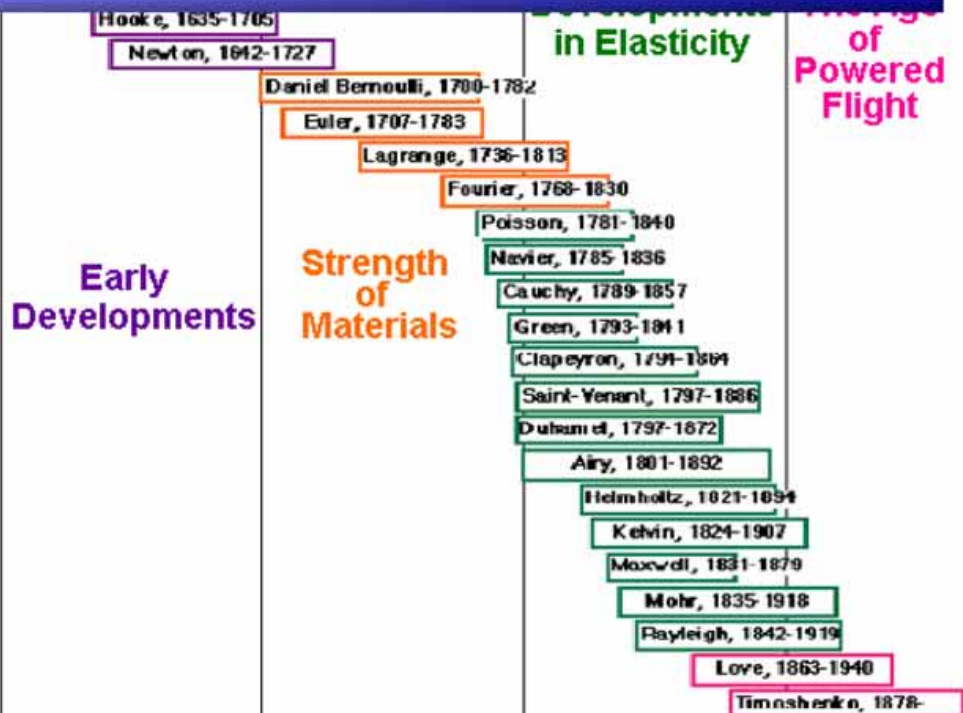
Stress concentrations

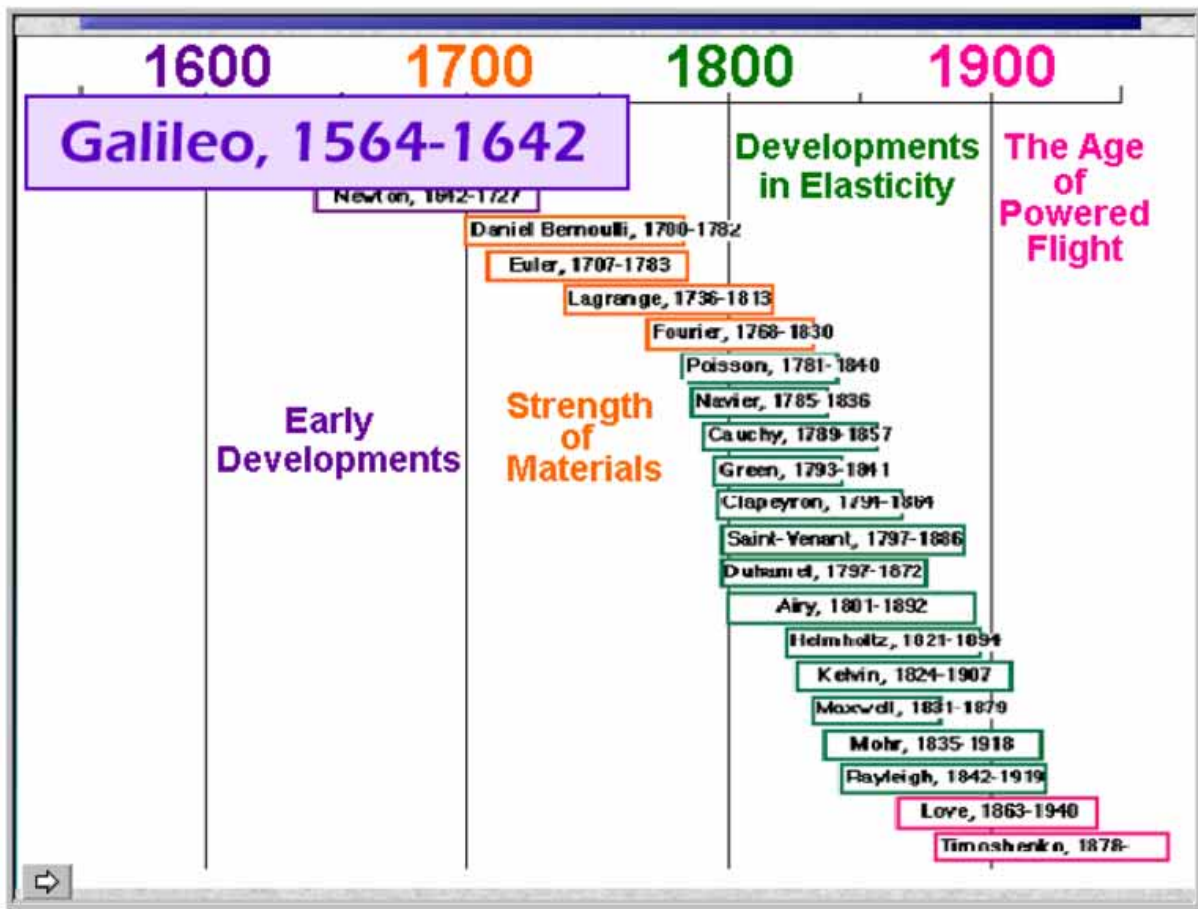
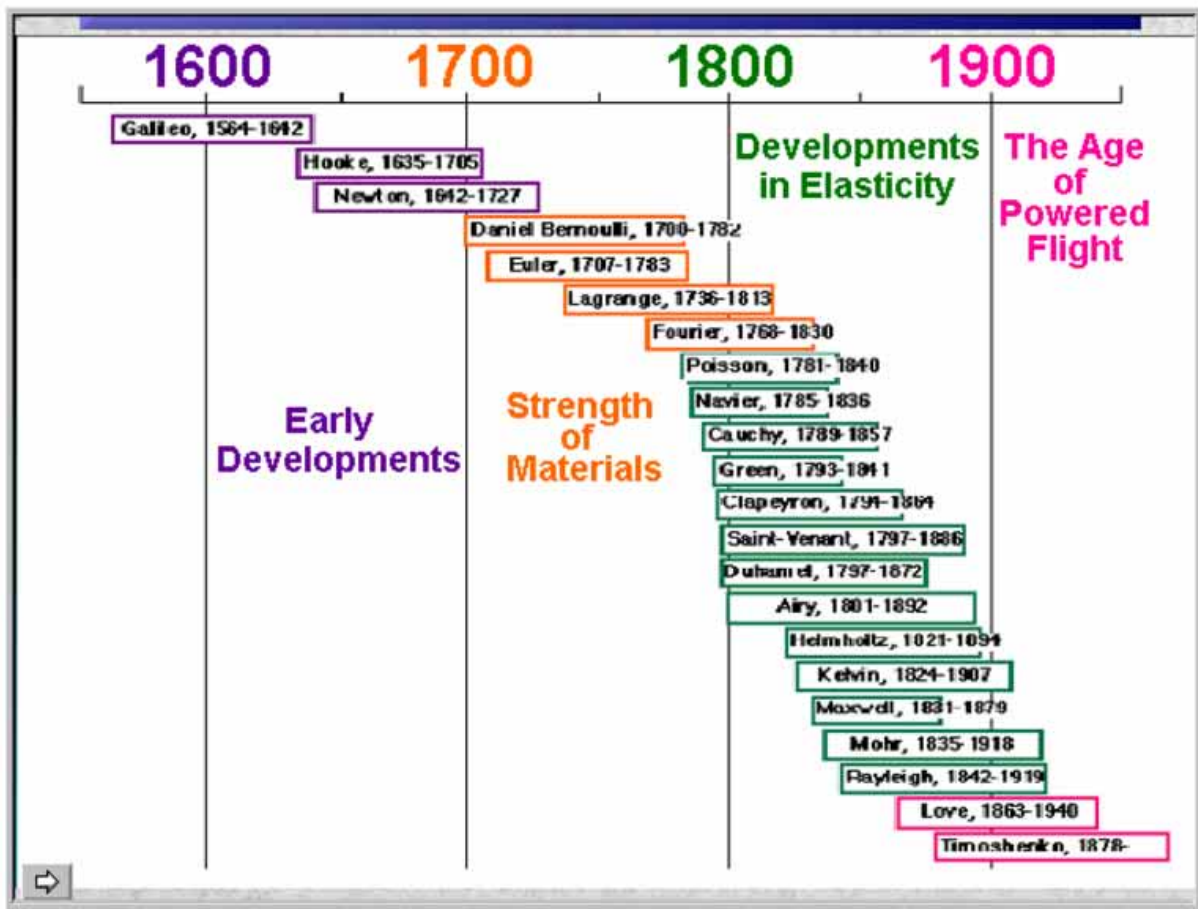


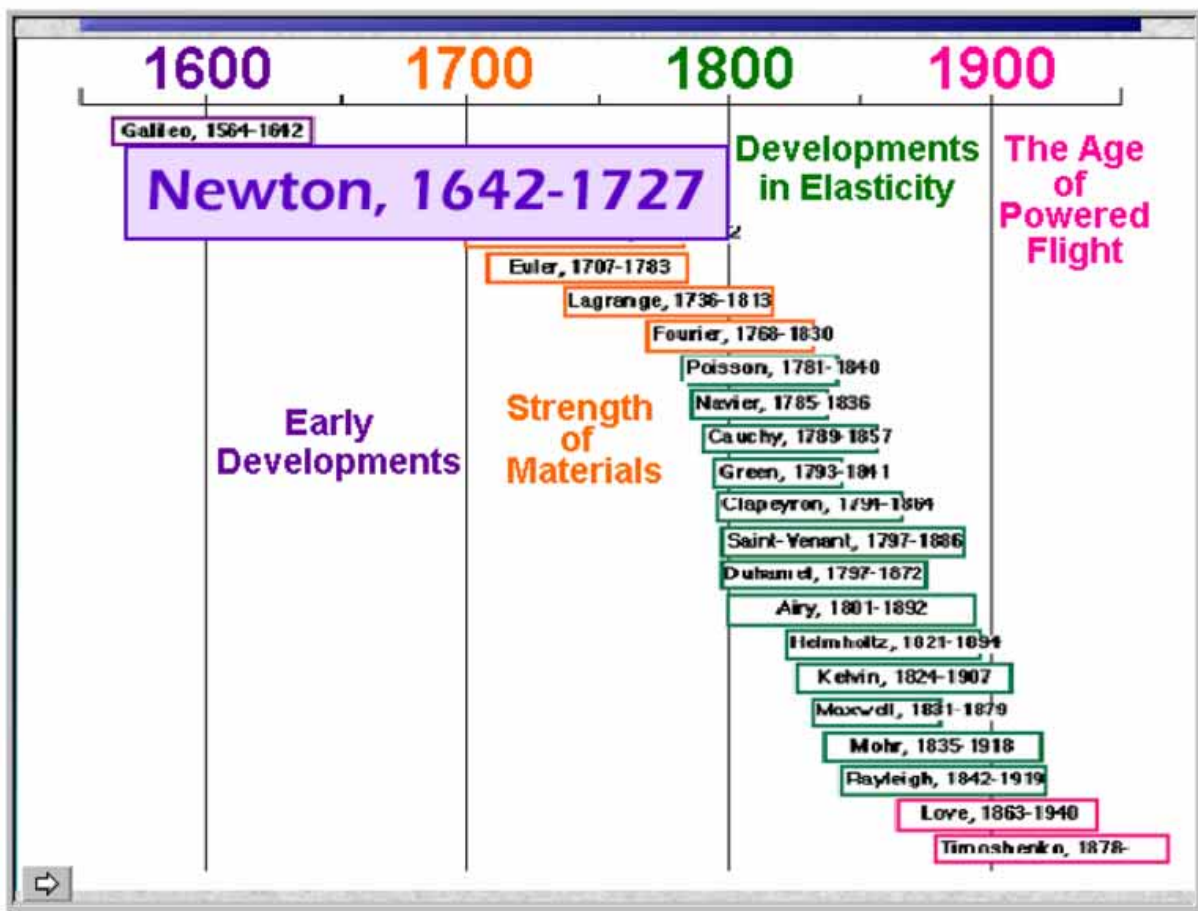
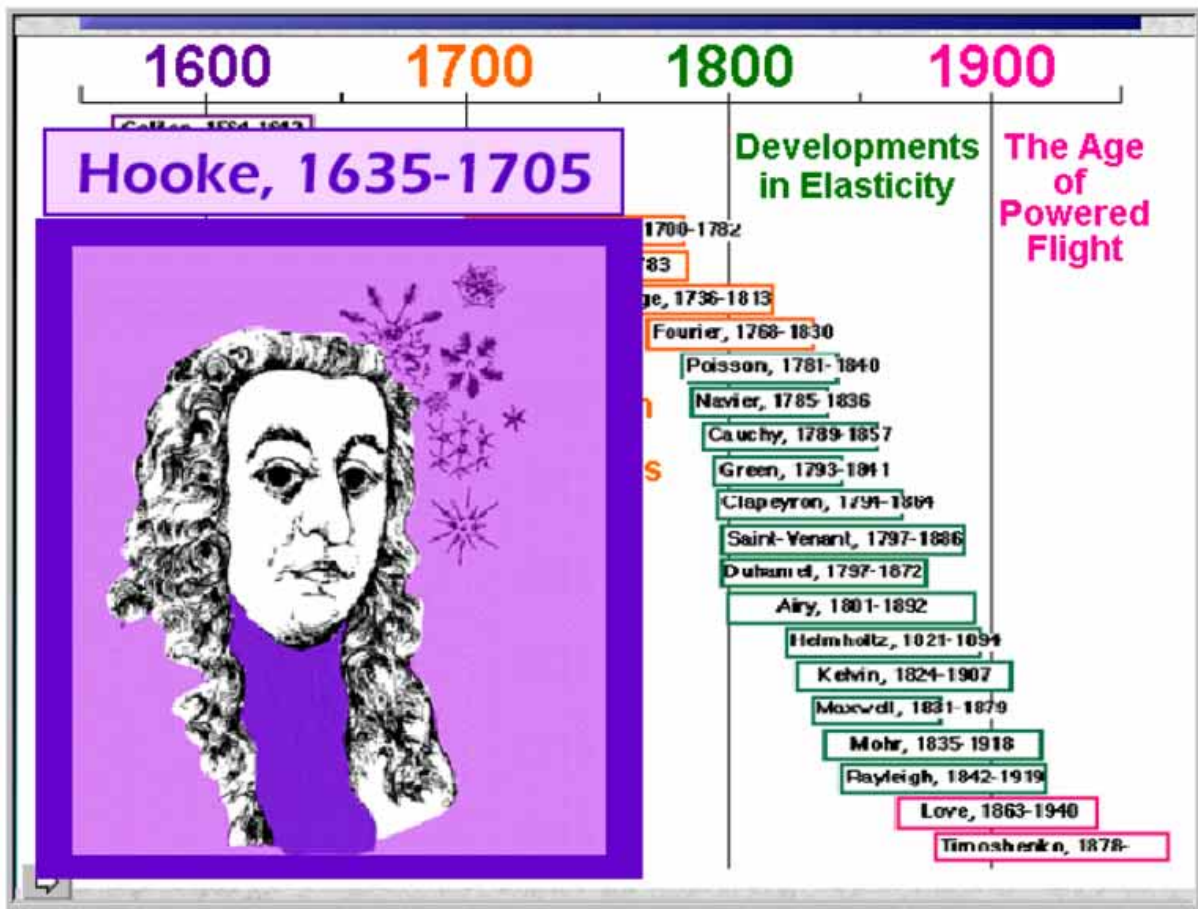
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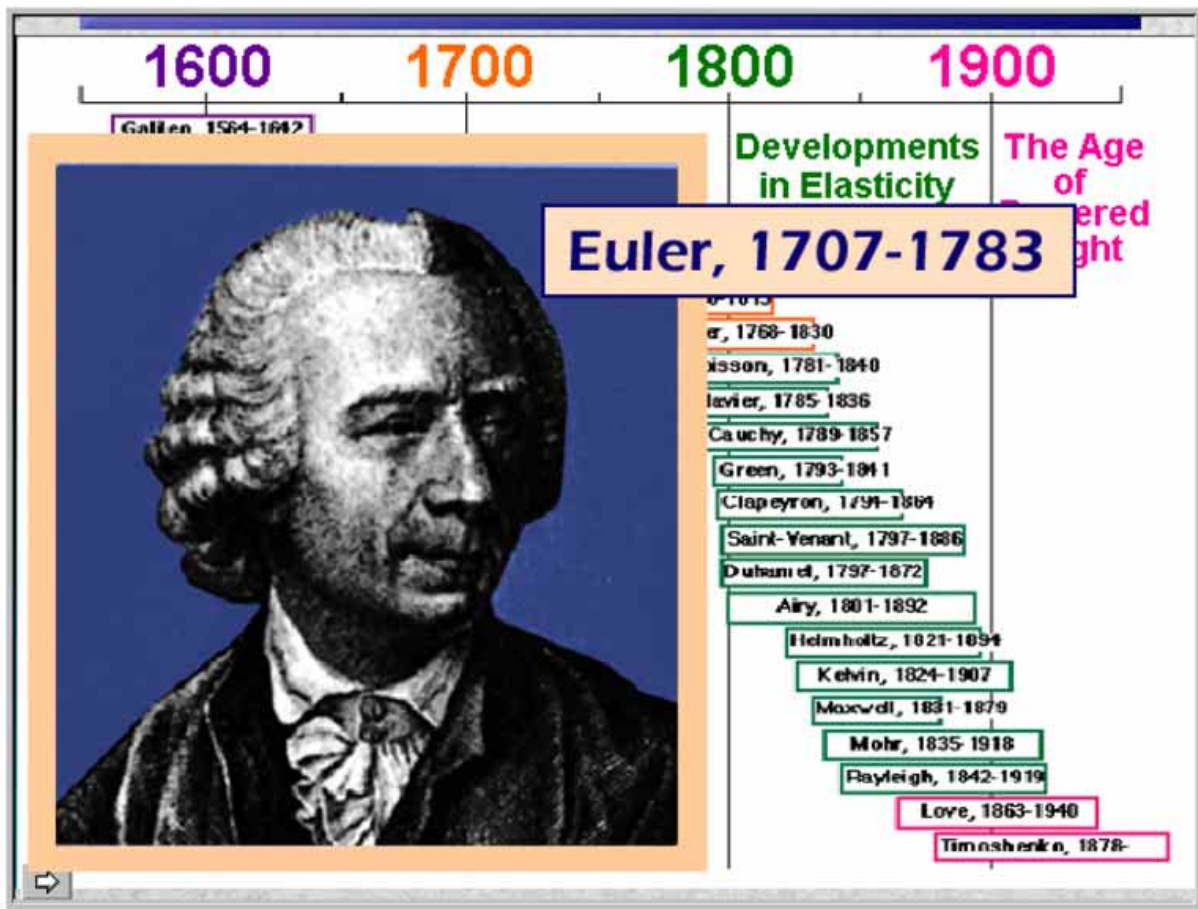
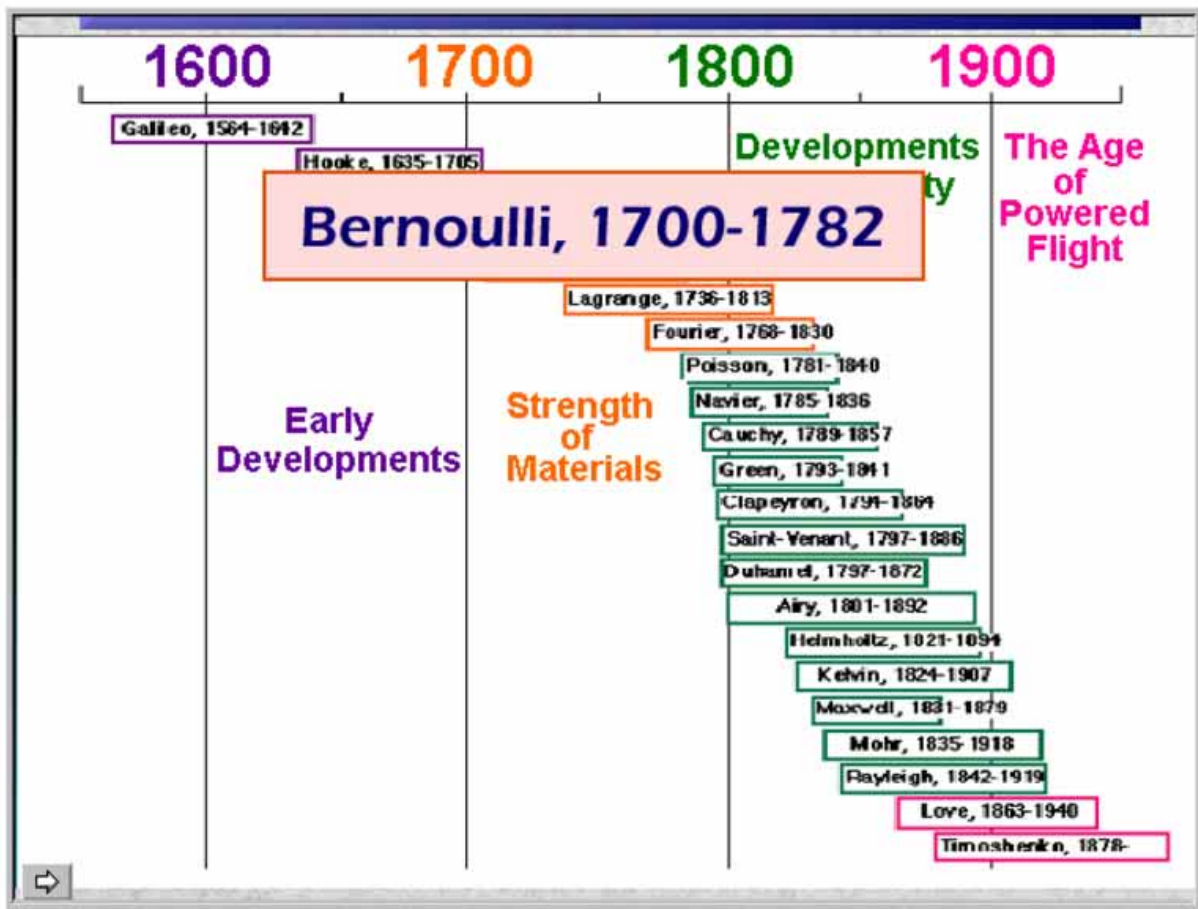


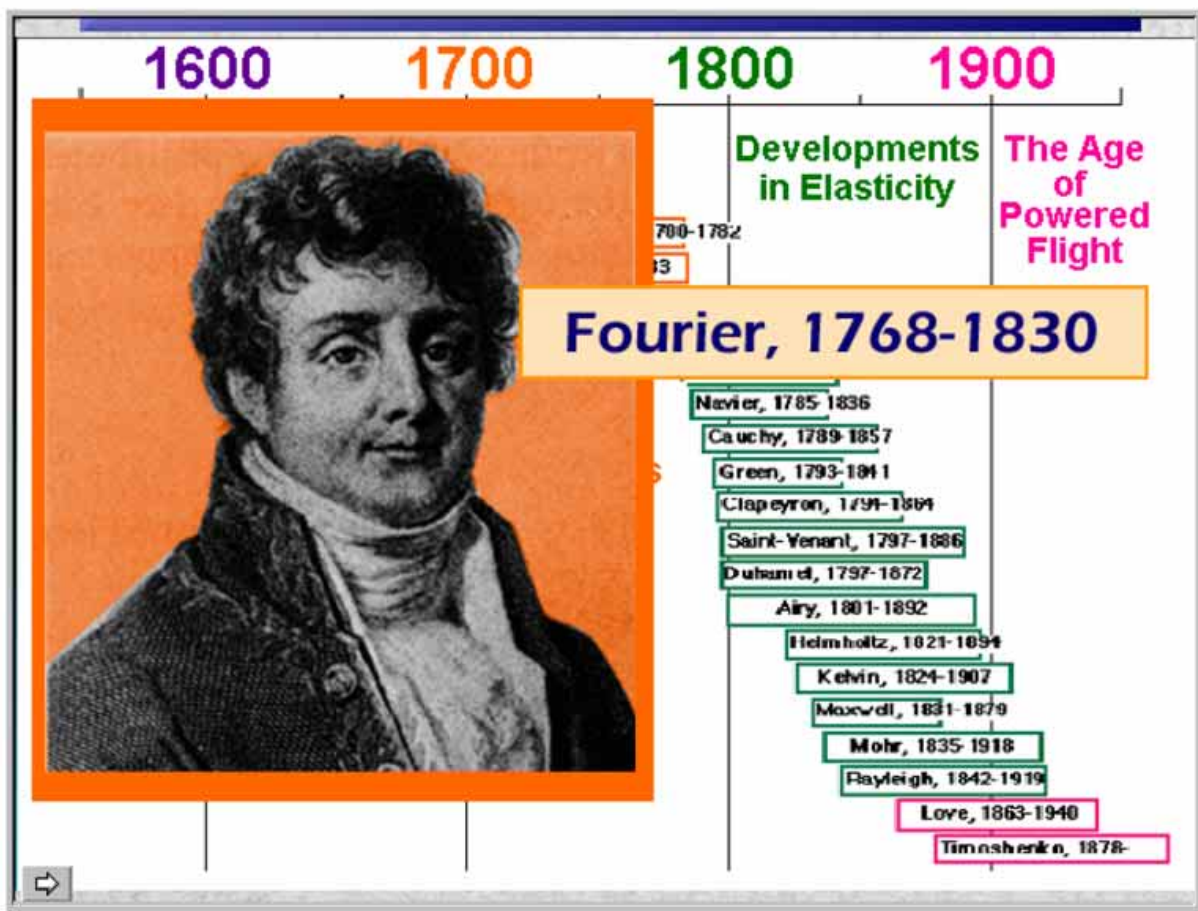
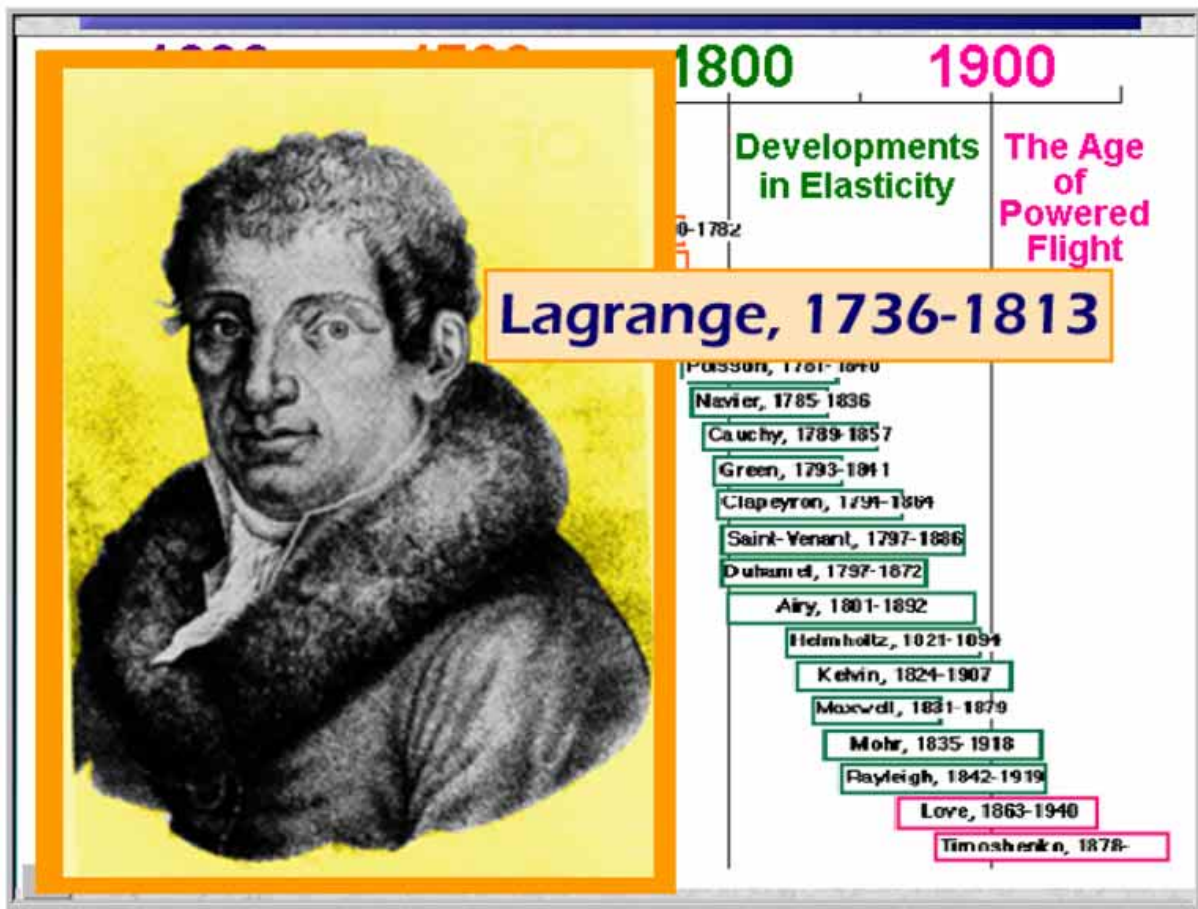
Brief History of the Development of Mechanics of Materials

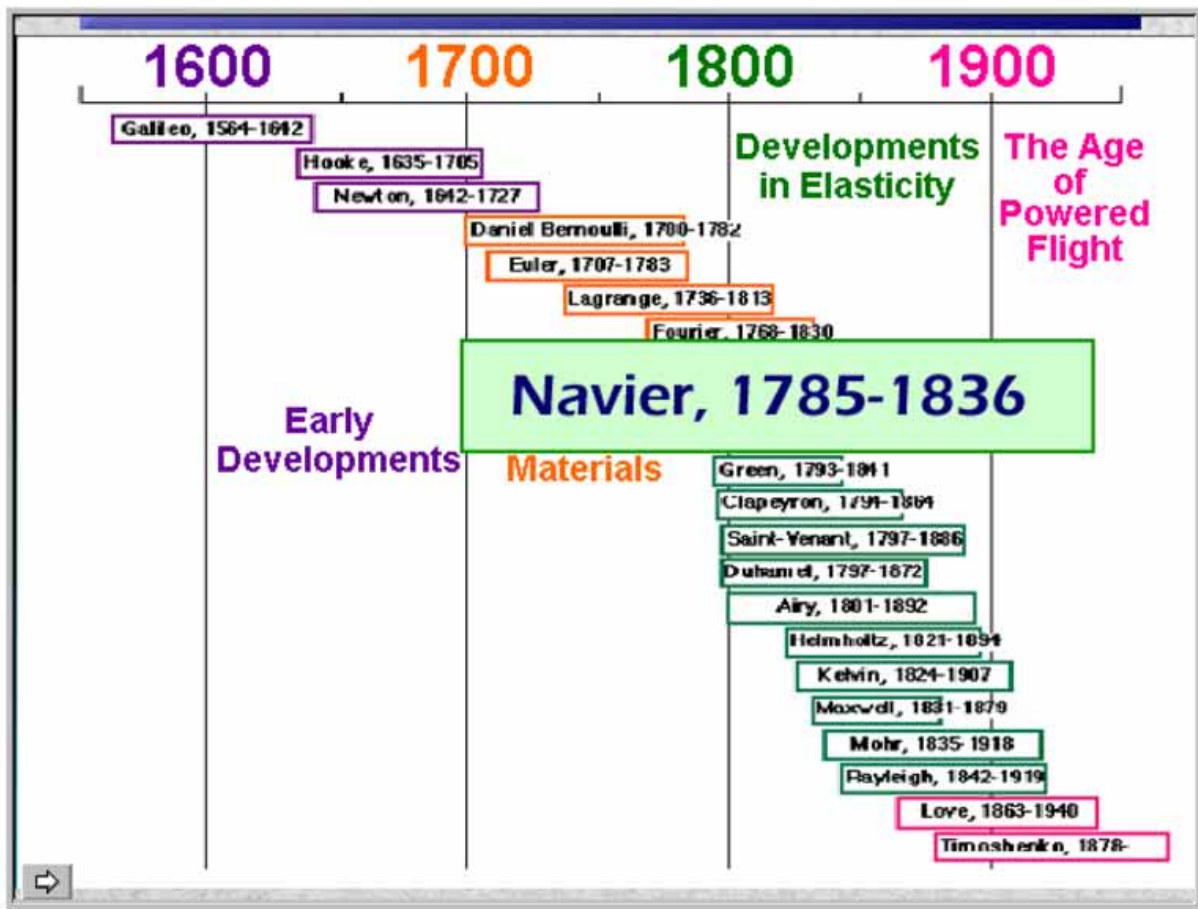
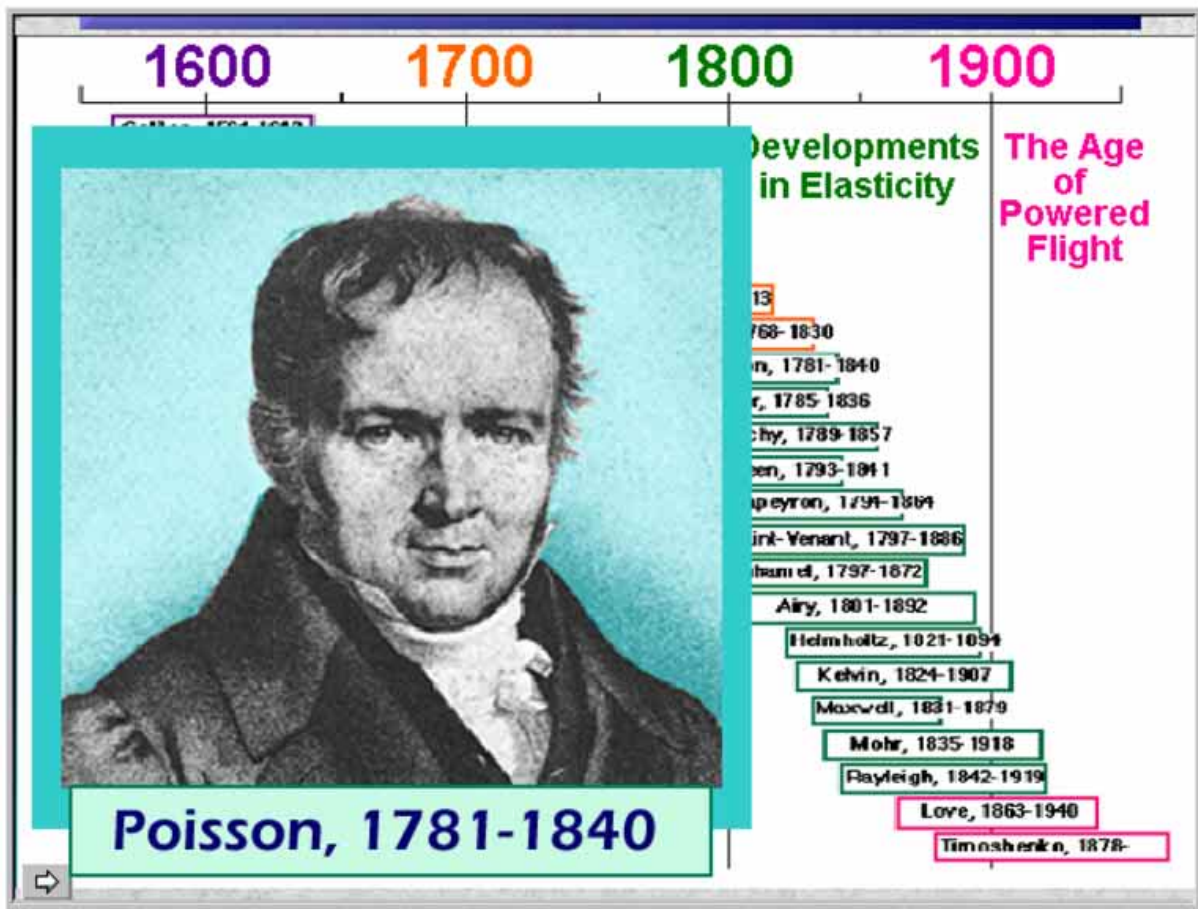


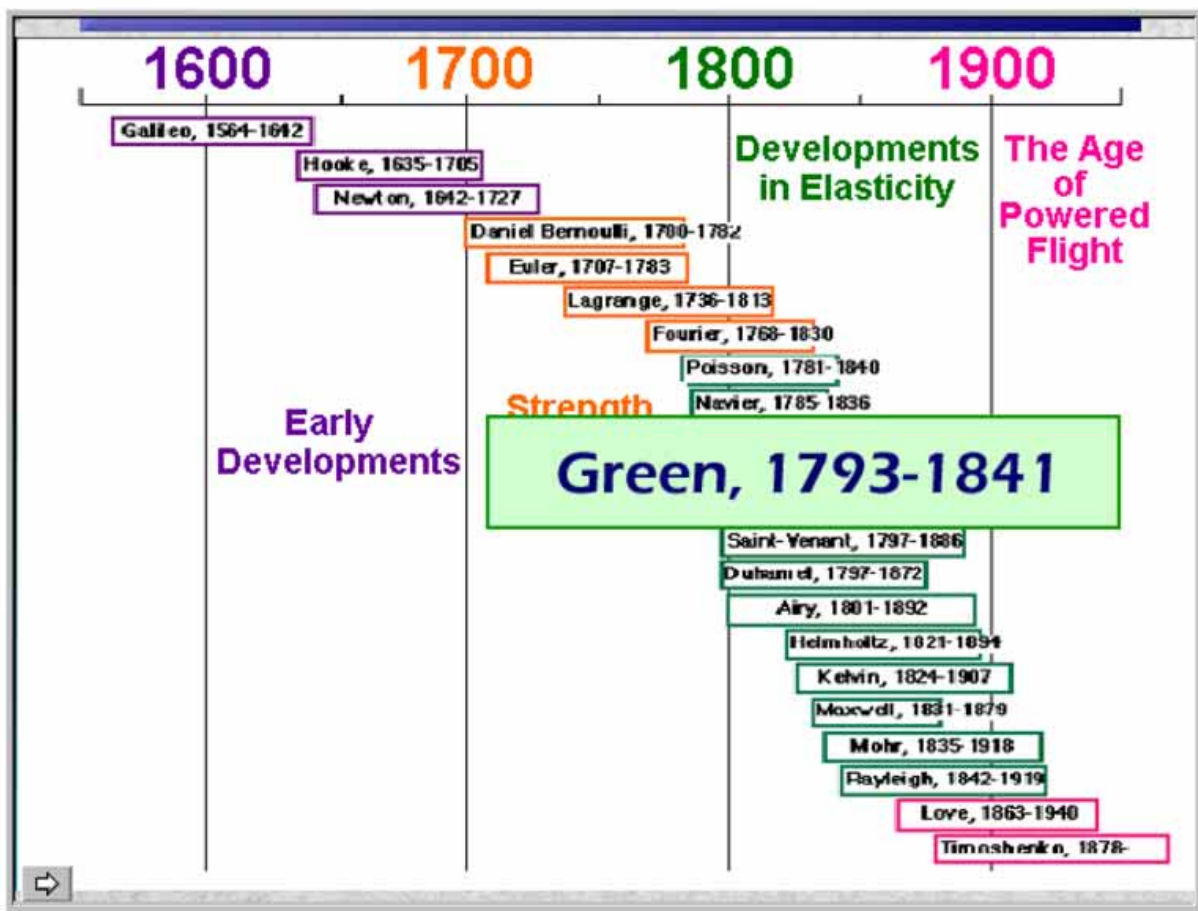
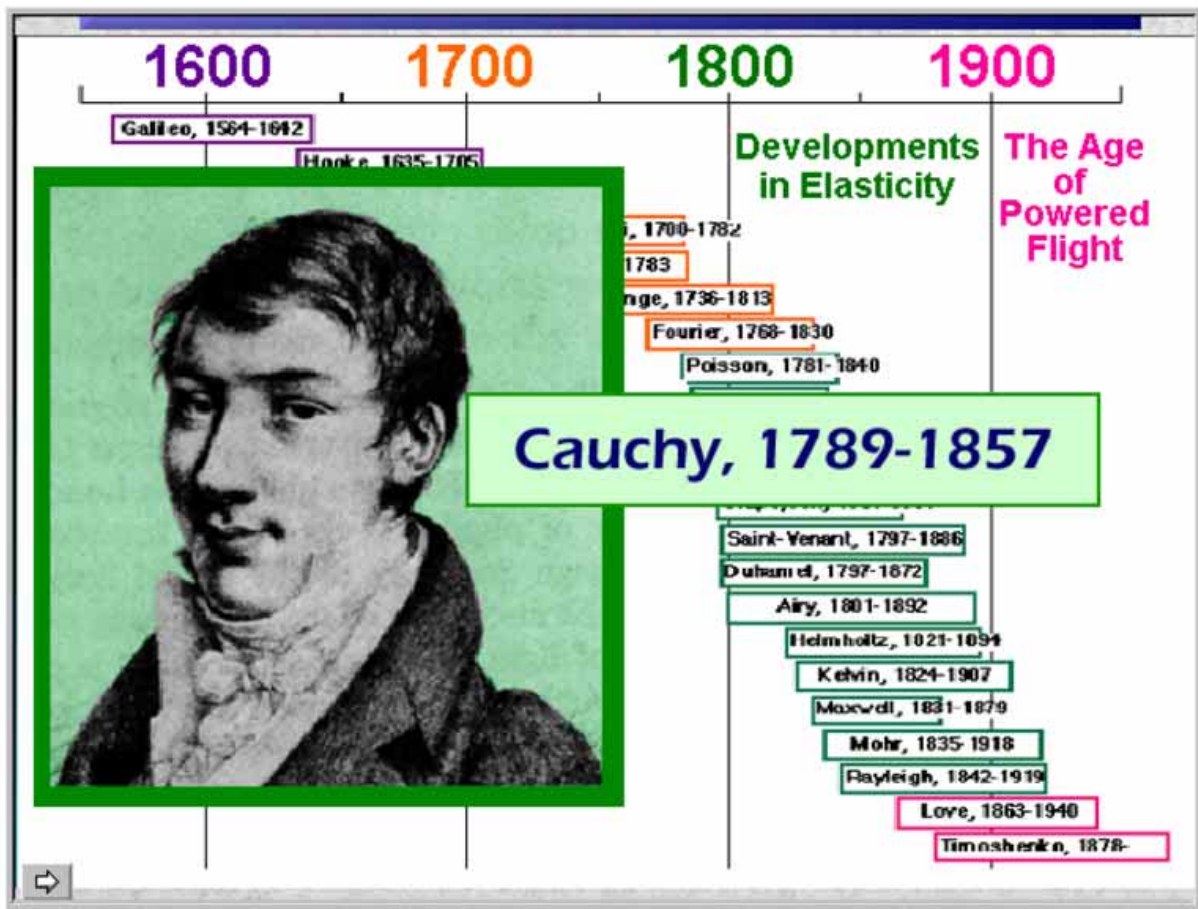


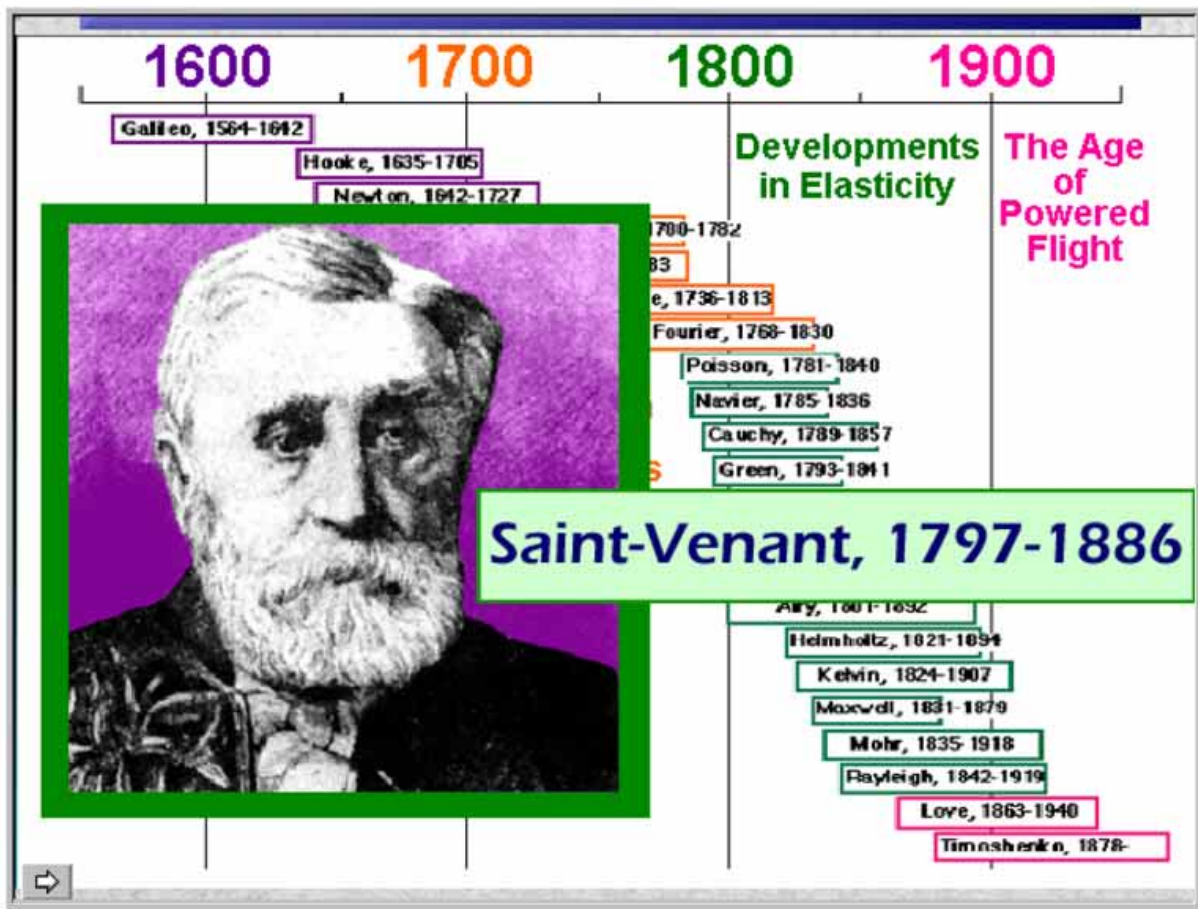
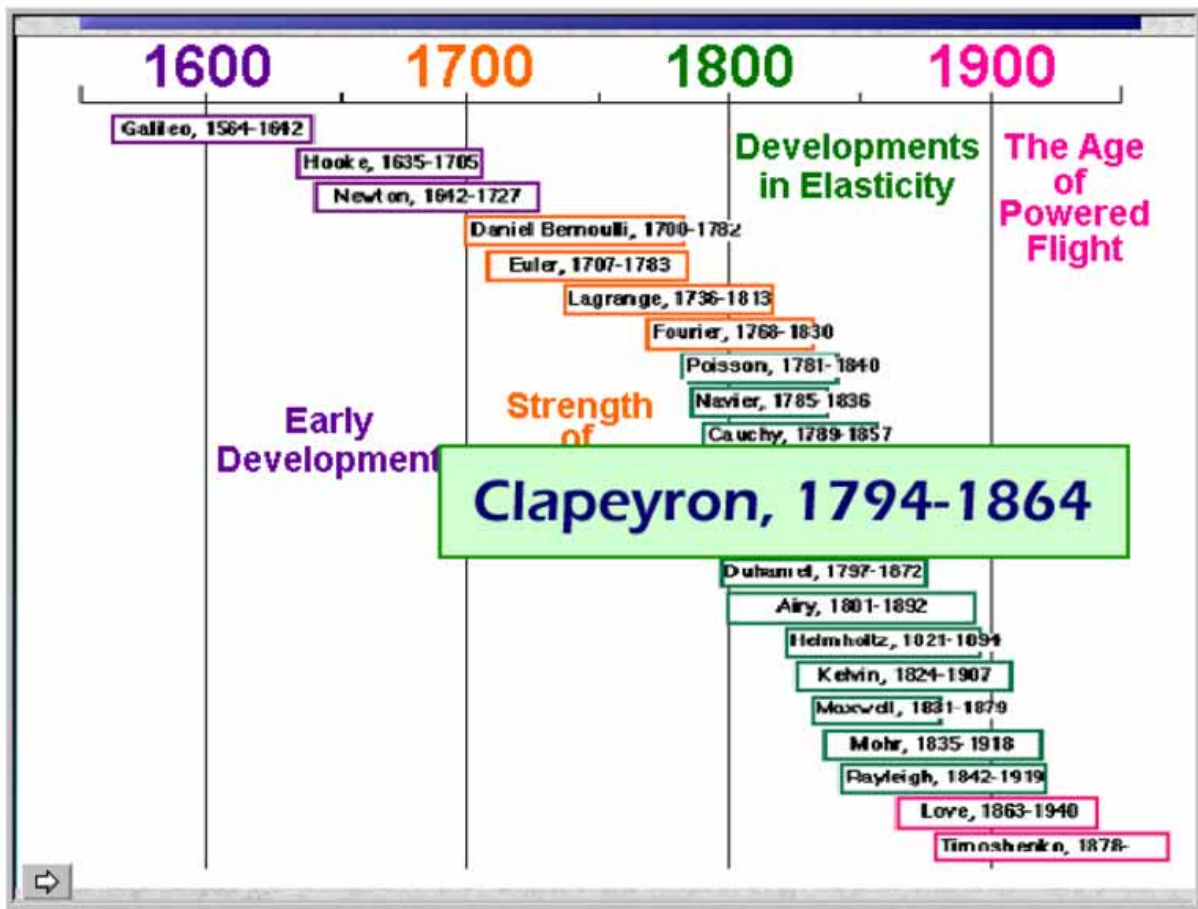


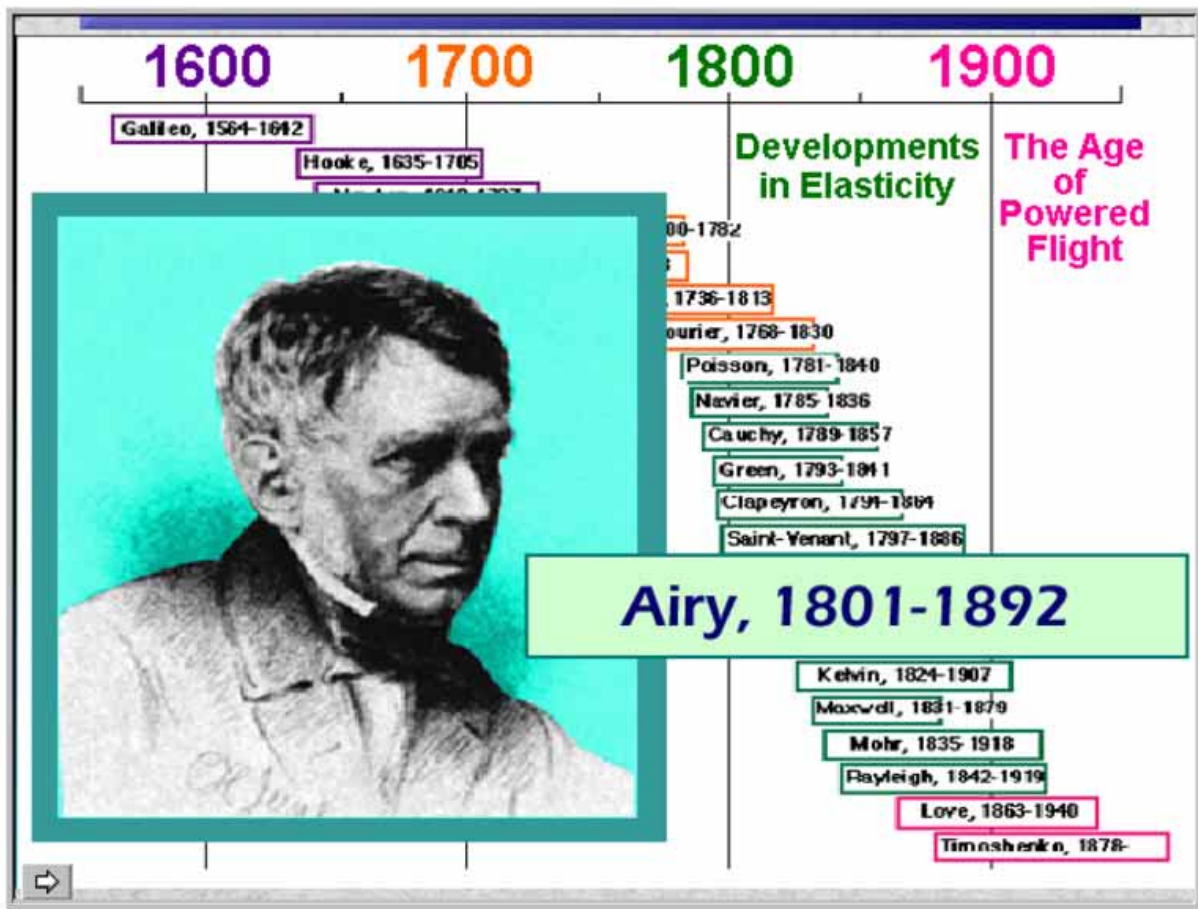
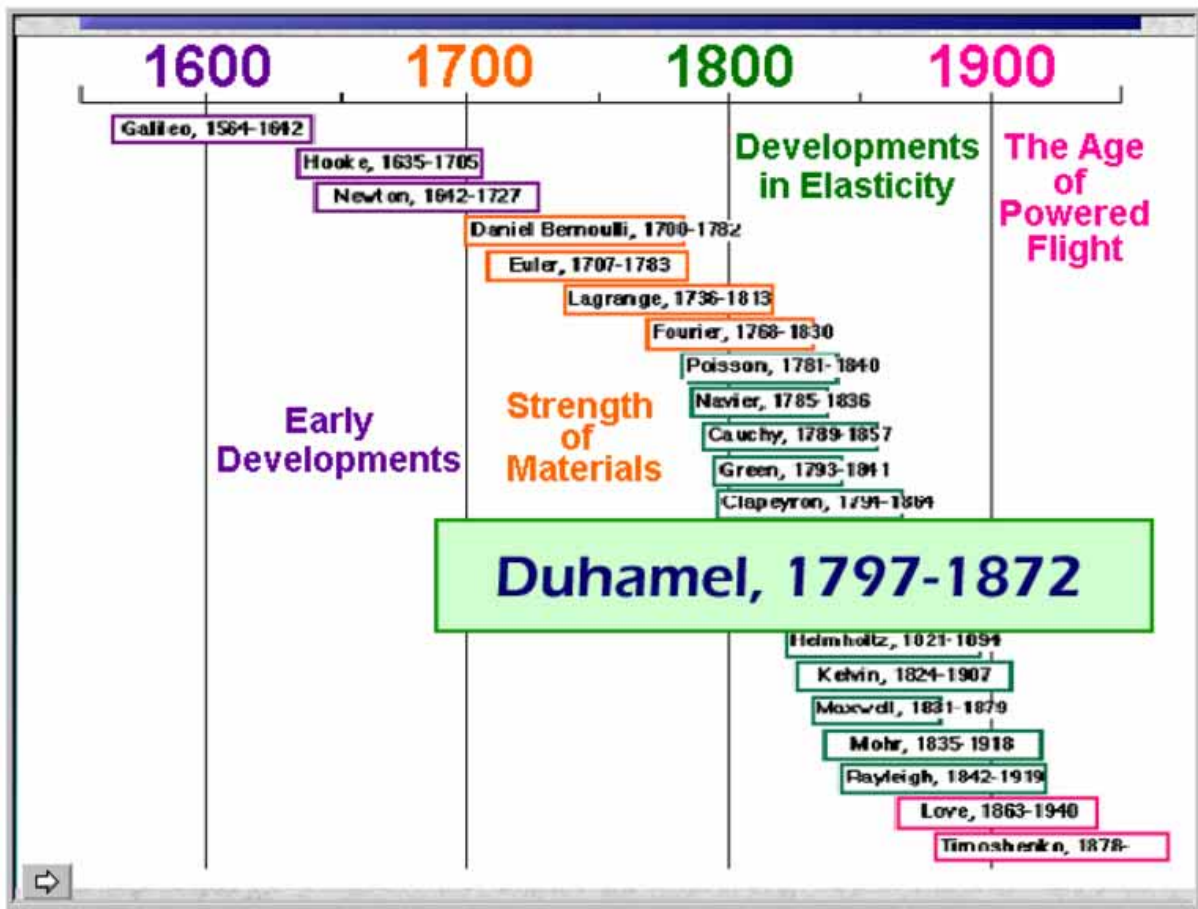


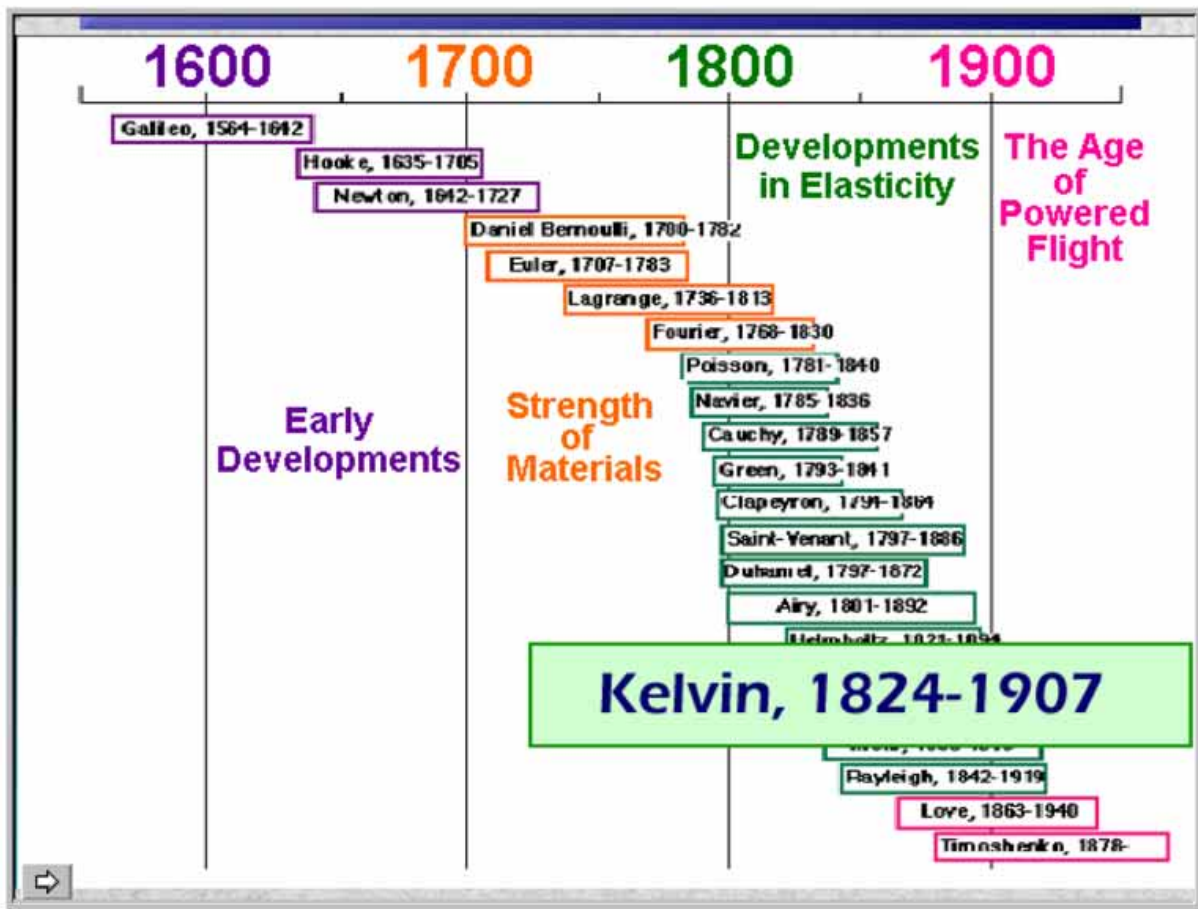
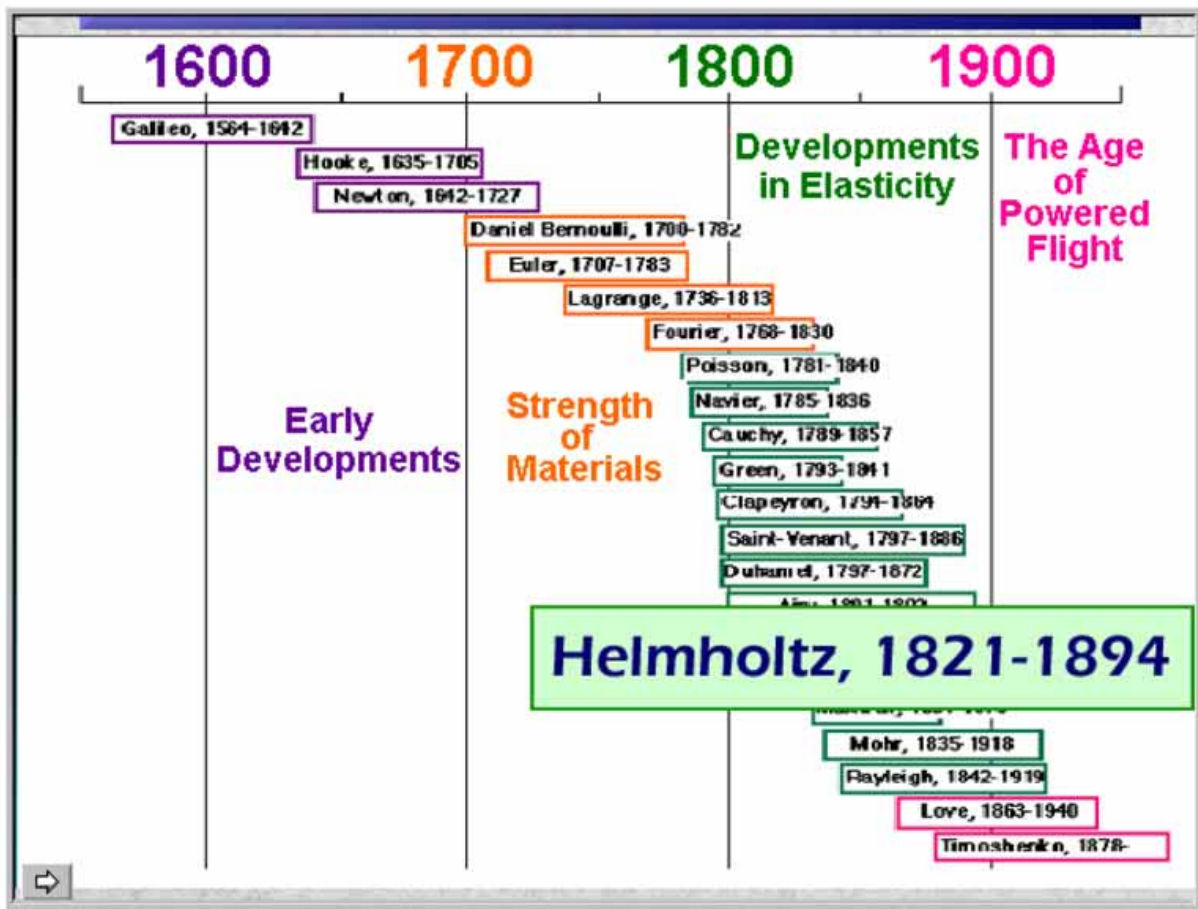


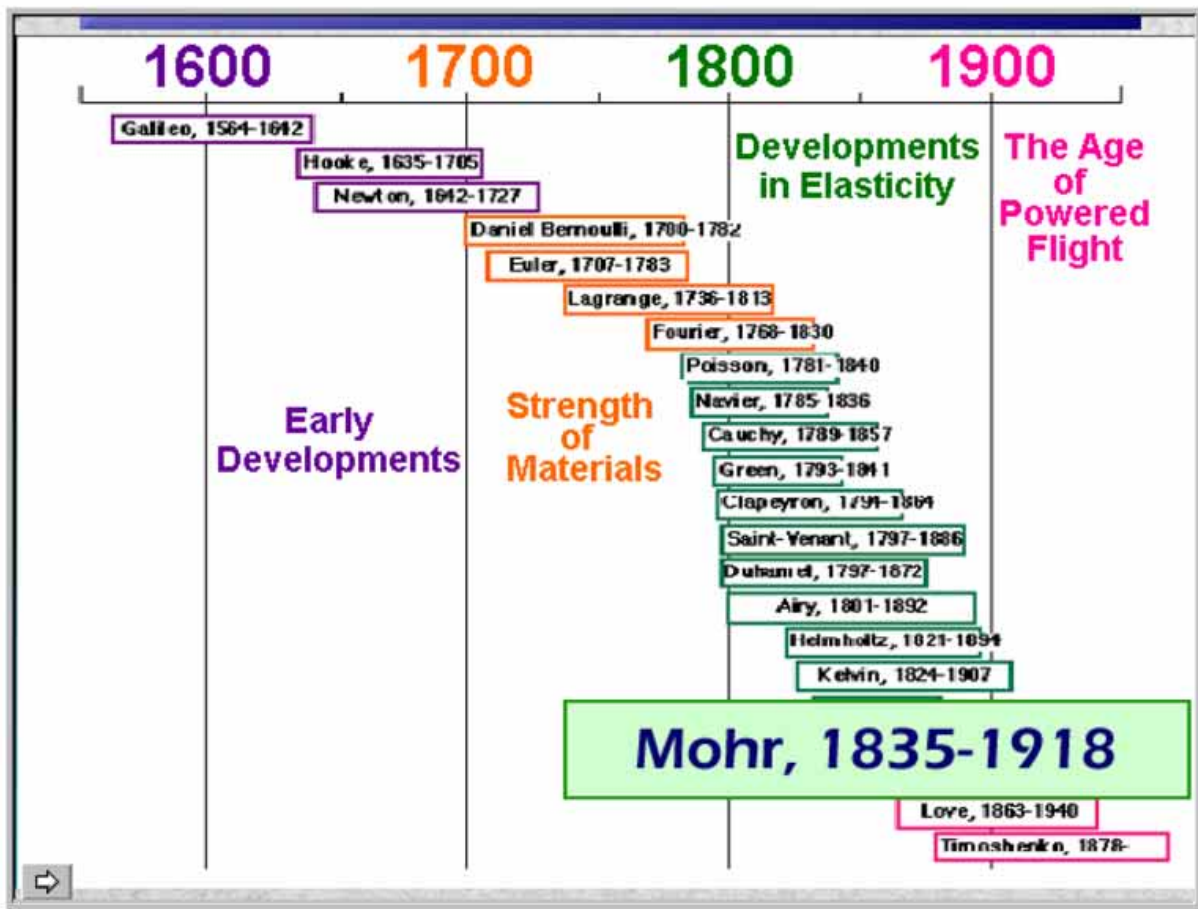
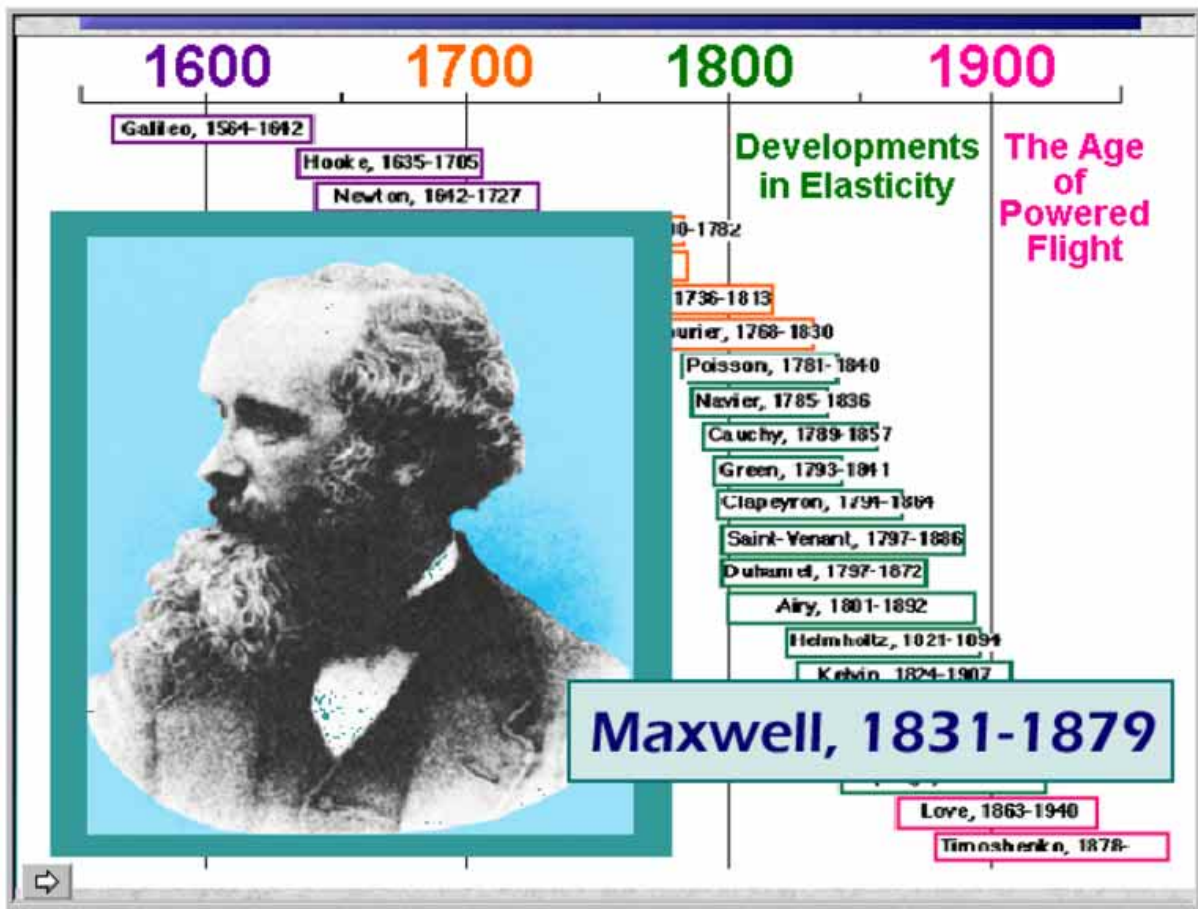


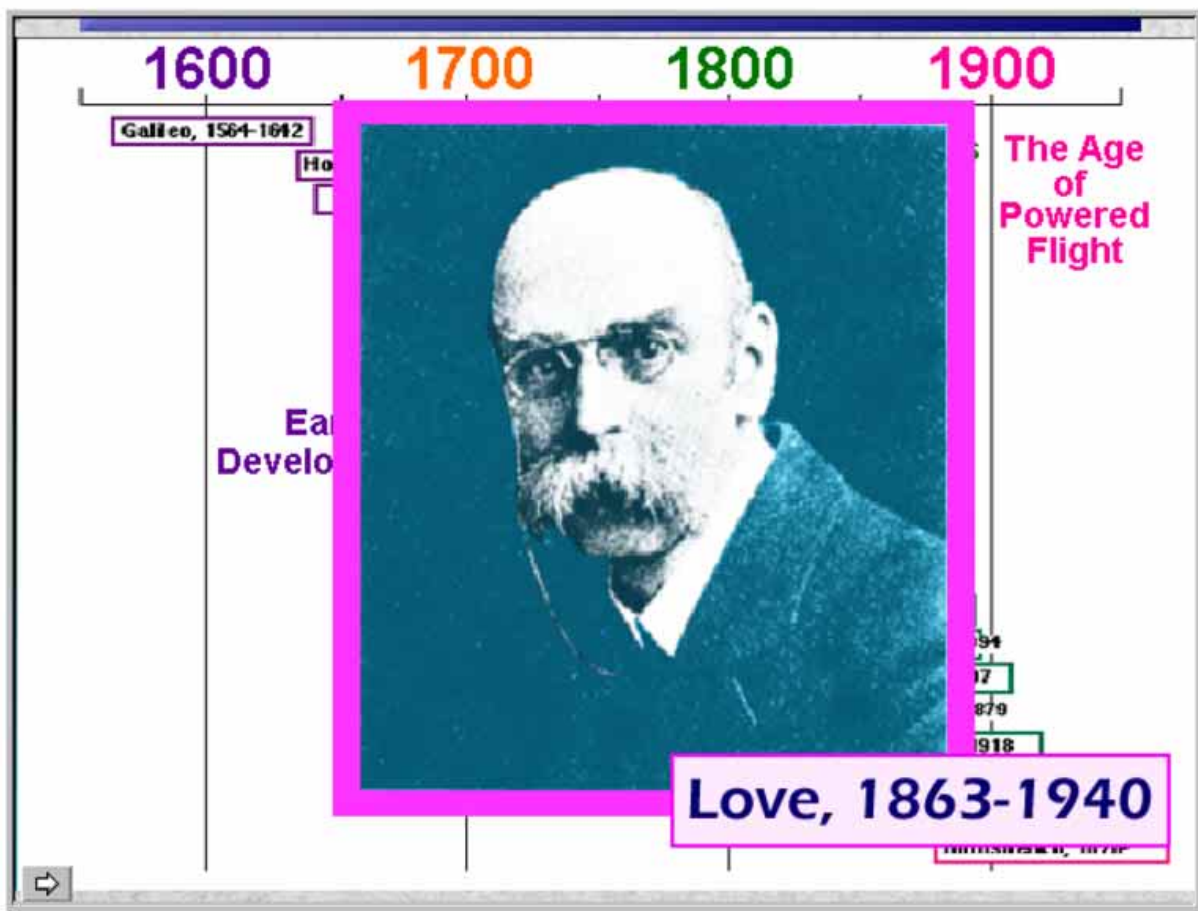
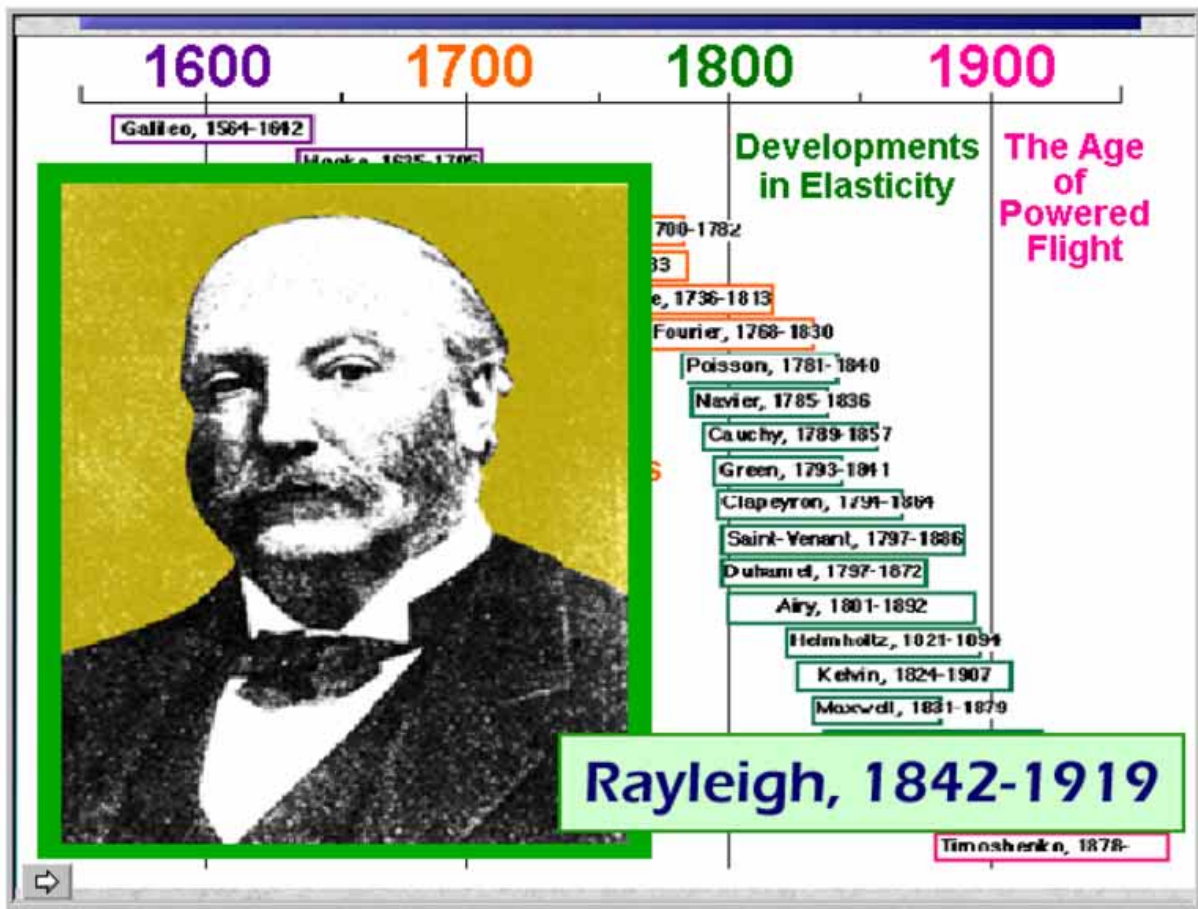


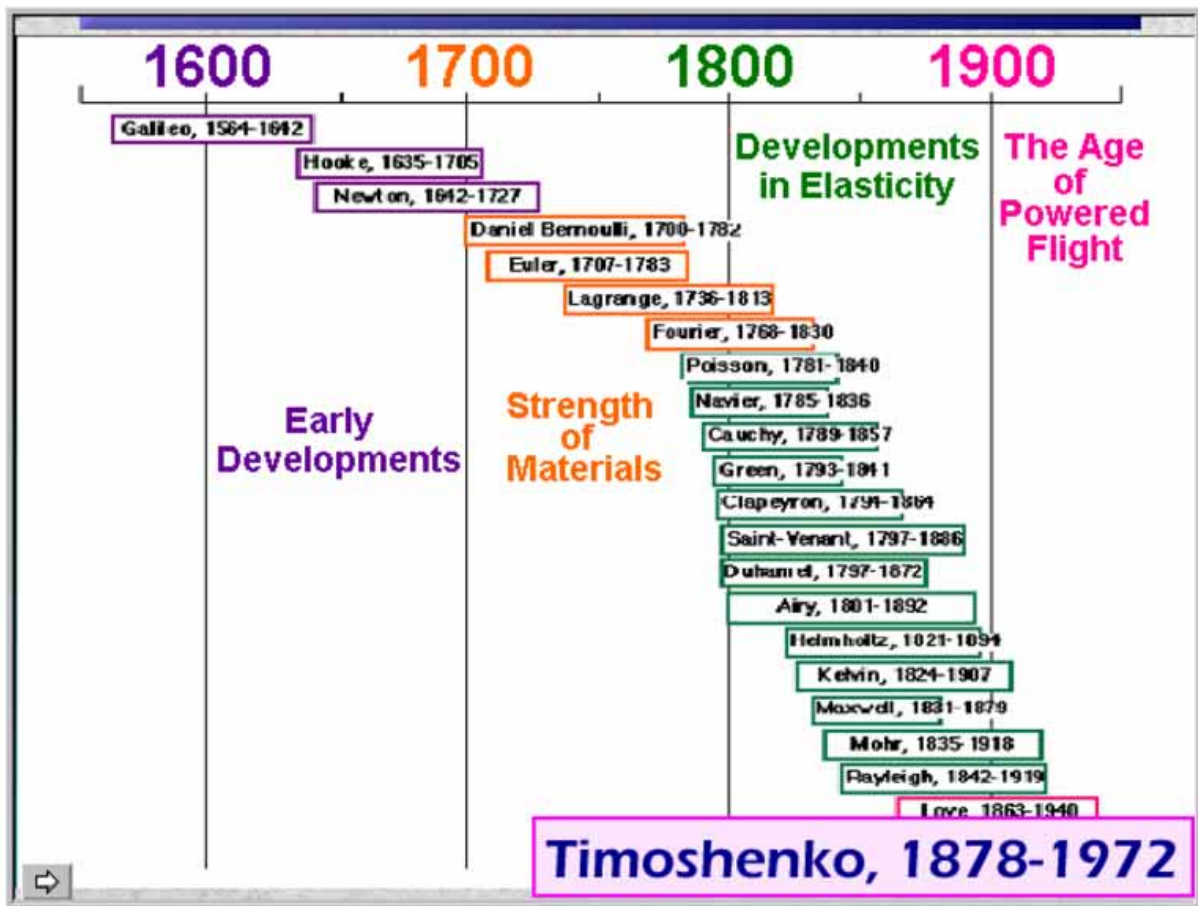
















Basic Assumptions in Mechanics of Materials

- 
Materials - continuous on macroscopic level
- 
Masses - reasonably large (small masses studied in quantum mechanics)
- 
Velocities - small compared to speed of light
- 
Simplifying assumptions usually made on kinematic and kinetic variables and material characteristics

Axioms of Nature

- They are **obeyed by all continuous bodies**, regardless of their shape or material makeup.
- They **cannot be proven** rigorously.
- They are **rarely**, if ever, observed to be **violated**.

Axioms of Nature

Kinetics

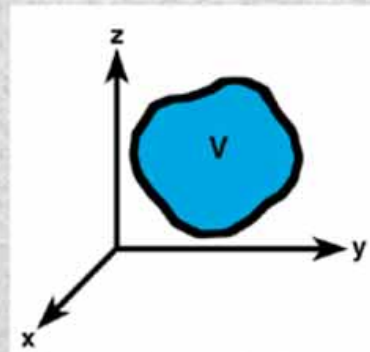
Branch of mechanics dealing with the motions of material bodies under the action of given forces.

Conservation of Mass

$$\frac{dm}{dt} = 0$$

$$m = \int_V \rho \, dV$$

ρ = mass density (mass per unit volume)



Axioms of Nature

Conservation of Momentum

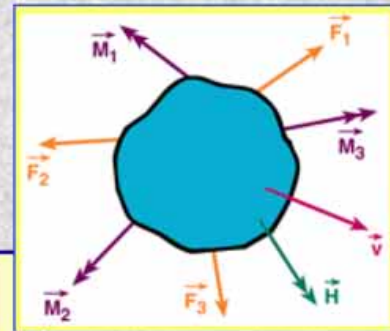
$$\Sigma \vec{F} = \frac{d}{dt} (m \vec{v}) , \quad \Sigma \vec{M} = \frac{d}{dt} (\vec{H})$$

$\vec{F} \equiv$ force vectors

$\vec{M} \equiv$ moment vectors

$\vec{v} \equiv$ velocity vector

$\vec{H} \equiv$ angular momentum vector



Axioms of Nature

Thermodynamics

Branch of physics dealing with the conservation of energy from one form to another.

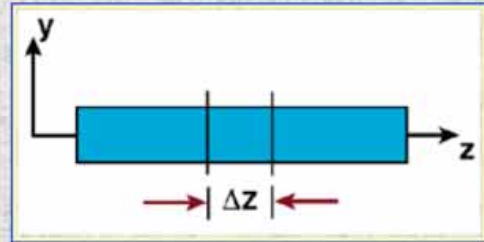
Conservation of Energy

Entropy Production

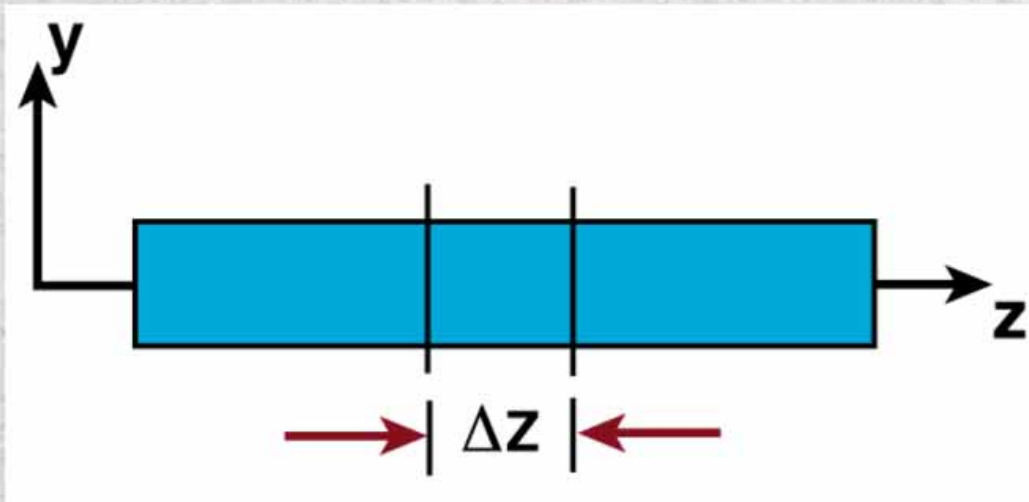
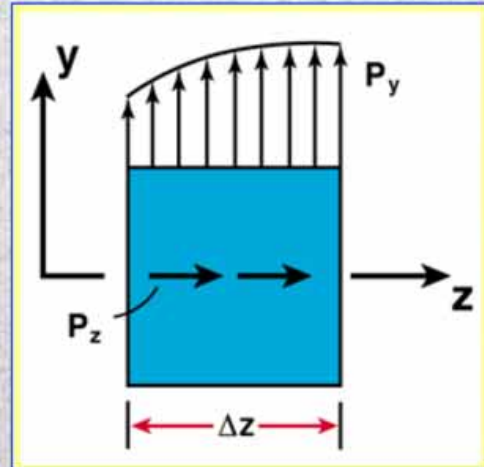
Planar Beams

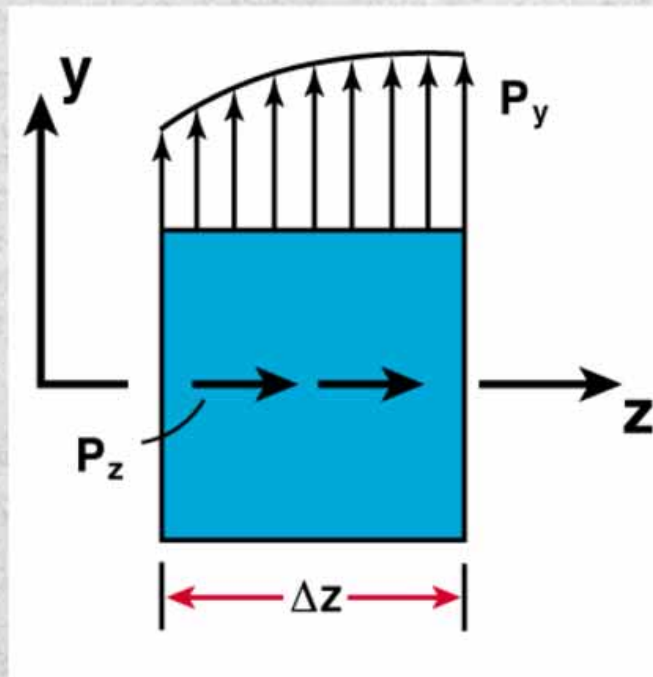
External Loading

p_y, p_z positive if acting in the positive y and z directions



p_y, p_z intensity of external loadings in the y and z directions



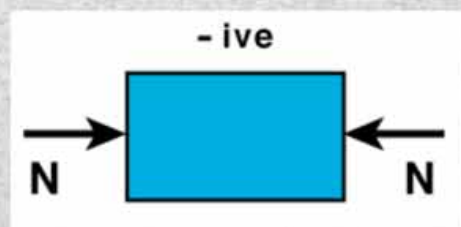
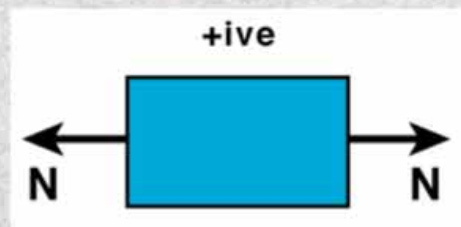


Planar Beams

Internal forces represent resistance to the relative motion of two adjacent cross sections.

Normal Force, N

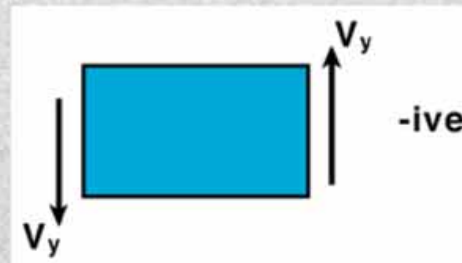
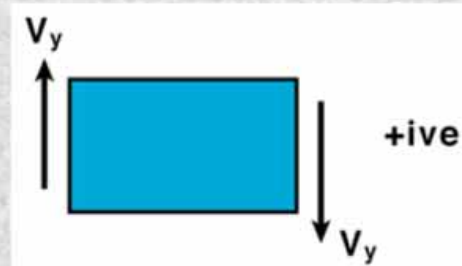
N positive if tensile
and negative if
compressive



Planar Beams

Shearing Force, V_y

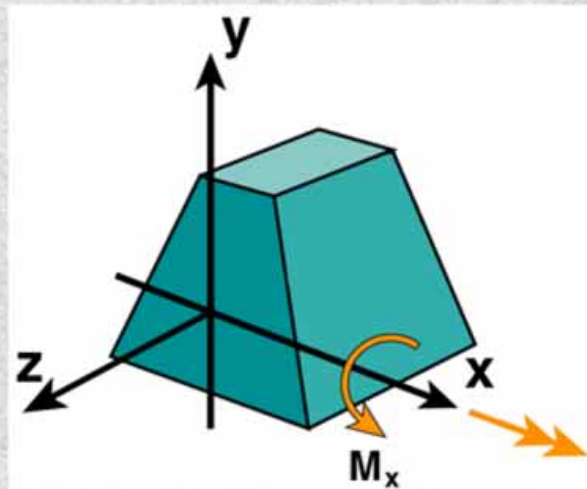
positive (+ive) and
negative (-ive) as
shown



Planar Beams

Bending Moment, M_x

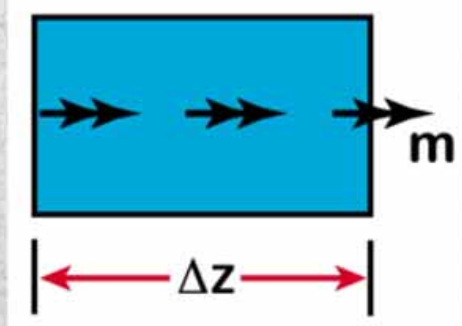
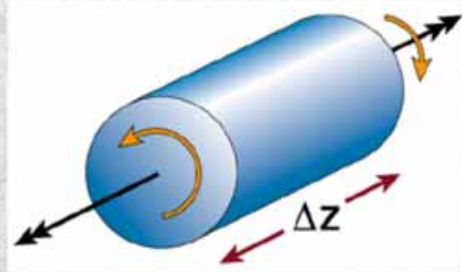
positive (+ive) and
negative (-ive) as
shown



Torsion of Circular Bars

External Twisting Moments

- Right hand screw rule used for representing moments.
- Intensity of external twisting moment m is positive, if its vector representation is in the positive coordinate direction



Torsion of Circular Bars

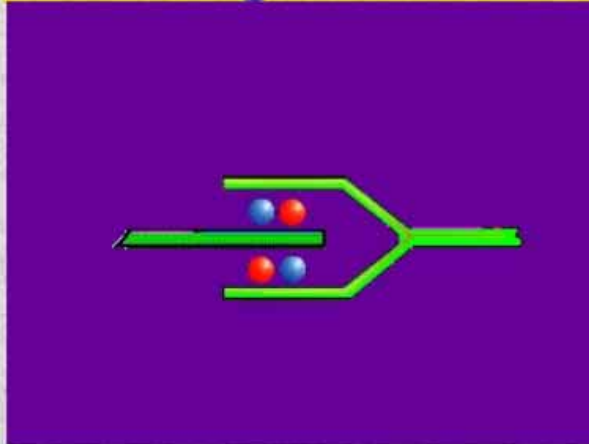
Internal Twisting Moments

positive (+ive) and negative (-ive) as shown



Intermediate Articulations (Hinges)

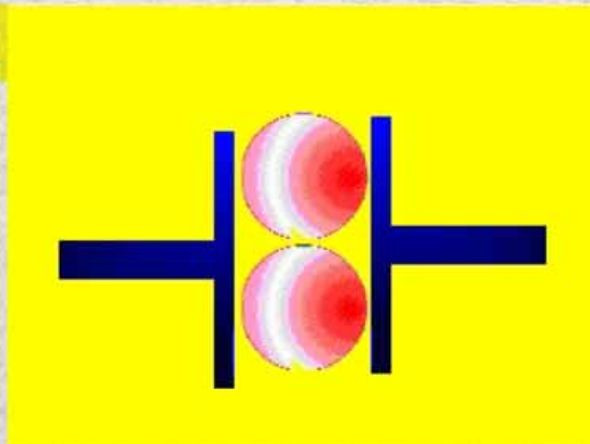
Axial hinge



Intermediate Articulations (Hinges)

Axial hinge

Shear hinge

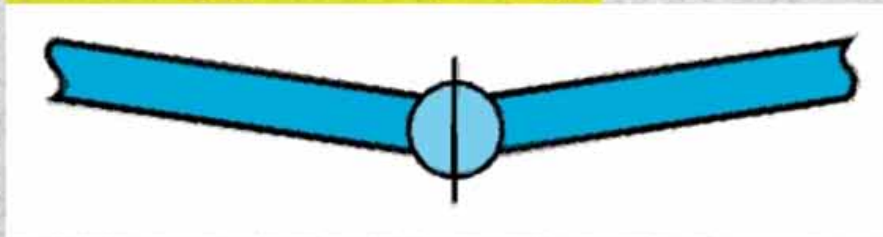


Intermediate Articulations (Hinges)

Axial hinge

Shear hinge

Bending (flexural) hinge

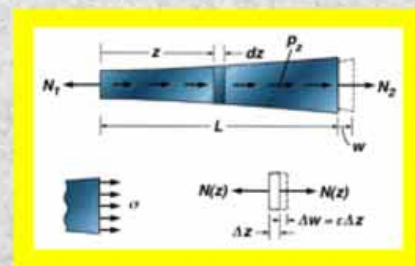


Elementary States of Stress and Strain

1. Axial Loading

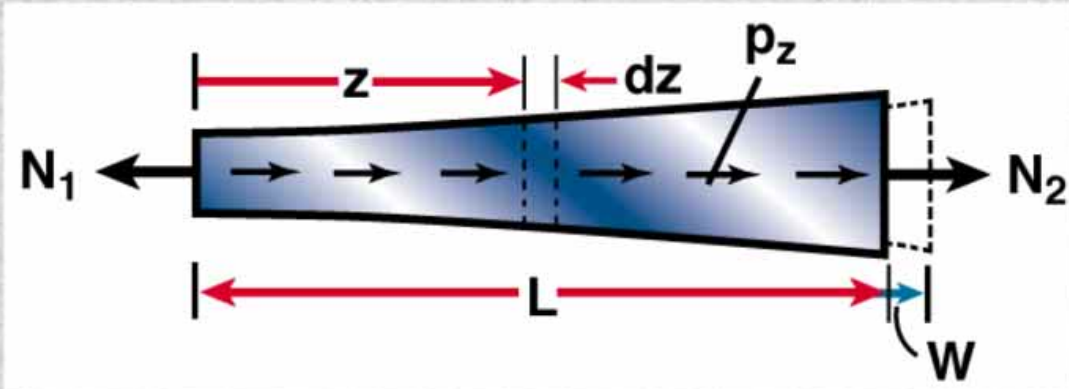
Geometry of Deformation

- Plane cross section remains plane and normal to the axis after deformation.



- Axial strain $\epsilon = dw/dz$ which is the same at all points of a given cross section.

Elementary States of Stress and Strain



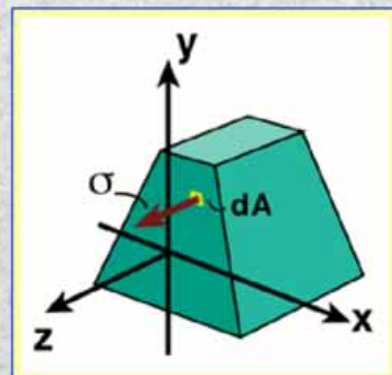
Elementary States of Stress and Strain

1. Axial Loading

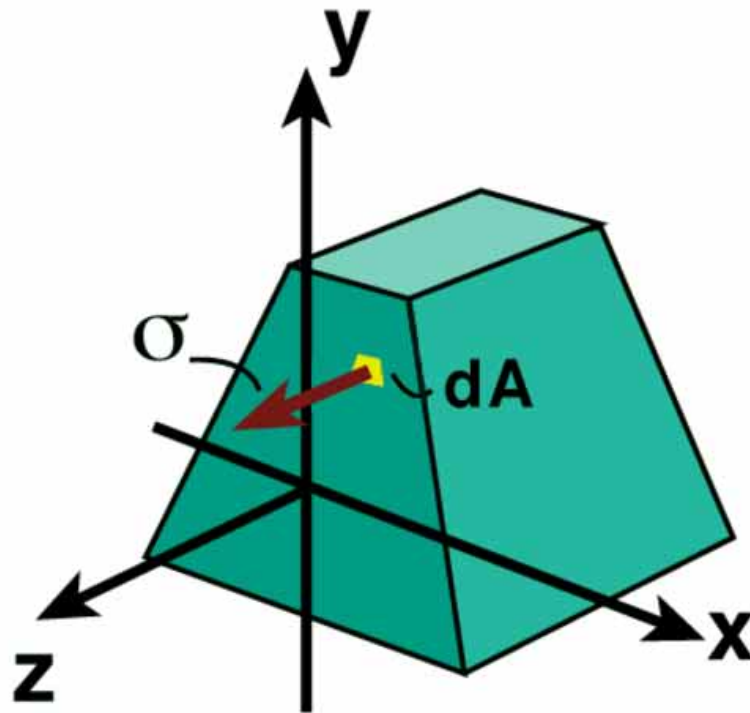
Static Relation - Equilibrium

$$\int_A \sigma \, dA = N$$

where σ = normal stress



Elementary States of Stress and Strain



Elementary States of Stress and Strain

1. Axial Loading

Constitutive Relation

$$\sigma = E \varepsilon$$

E = modulus of elasticity in tension or compression

$$\int_A E \varepsilon dA = N$$

If E is uniform at all points of the cross section, then

$$E \varepsilon \int_A dA = N$$

Elementary States of Stress and Strain

1. Axial Loading

Constitutive Relation

$$\int_A E \varepsilon dA = N$$

If E is uniform at all points of the cross section, then

$$E \varepsilon \int_A dA = N$$

or

$$E \varepsilon A = N$$

Elementary States of Stress and Strain

1. Axial Loading

Constitutive Relation

$$E \varepsilon \int_A dA = N$$

or $E \varepsilon A = N$

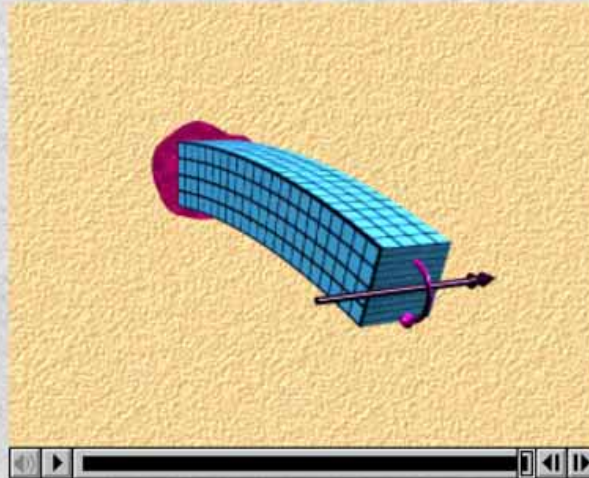
$$\sigma = N/A$$

$$\varepsilon = \frac{dw}{dz} = \frac{N}{EA} \text{ and } EA = \text{extensional stiffness}$$

Elementary States of Stress and Strain

2. Pure and Transverse Bending

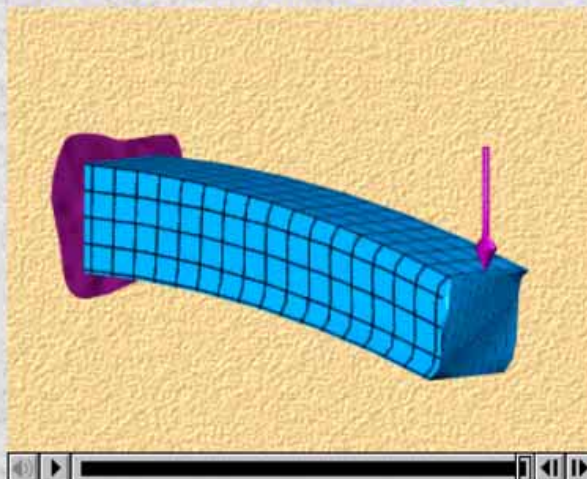
Pure bending refers to bending moment only (no axial force, shear force, or twisting moment)



Elementary States of Stress and Strain

2. Pure and Transverse Bending

Transverse bending refers to combination of bending moment and shearing force

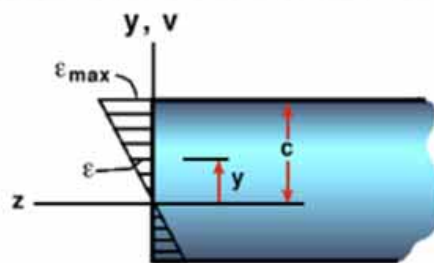
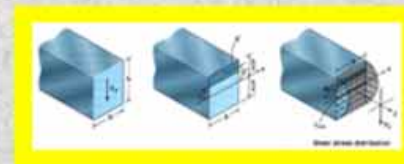
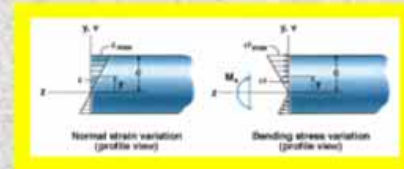


Elementary States of Stress and Strain

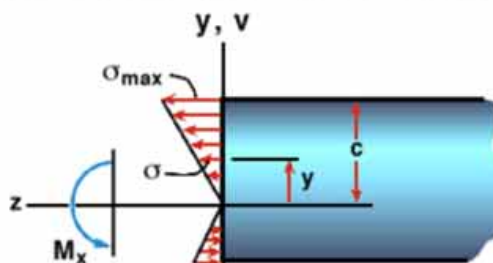
2. Pure and Transverse Bending

Geometry of Deformation

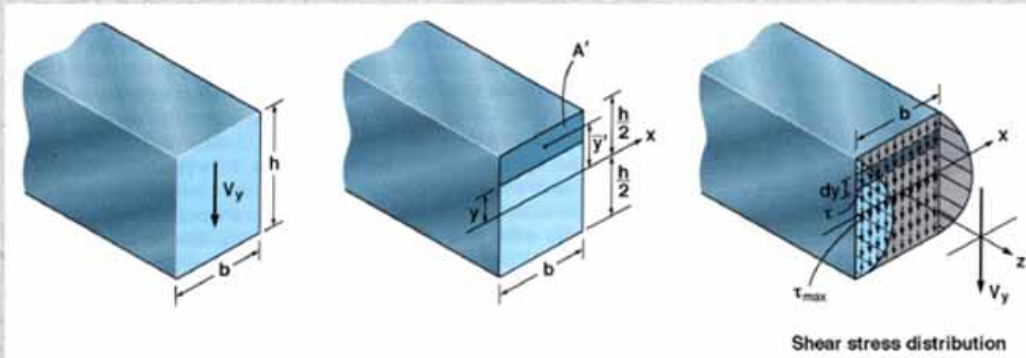
- Plane cross section before bending remains plane after bending and normal to the center line of the beam.
- The neutral axis is the x-axis.



Normal strain variation
(profile view)



Bending stress variation
(profile view)



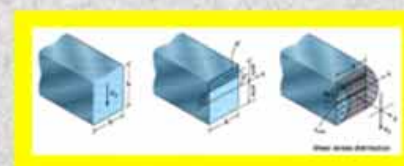
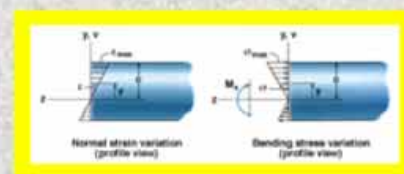
Elementary States of Stress and Strain

2. Pure and Transverse Bending

Geometry of Deformation

$$\kappa = - \frac{d^2 v}{dz^2}$$

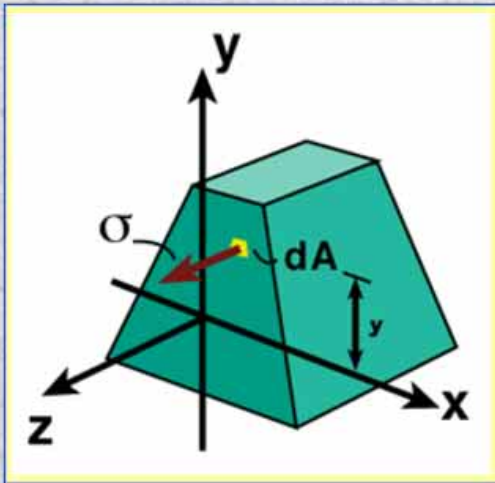
$$\epsilon = y \times \kappa = - y \frac{d^2 v}{dz^2}$$



Elementary States of Stress and Strain

2. Pure and Transverse Bending

Static Relation - Equilibrium



$$\int_A \sigma y dA = M_x$$

Elementary States of Stress and Strain

2. Pure and Transverse Bending

Constitutive Relation

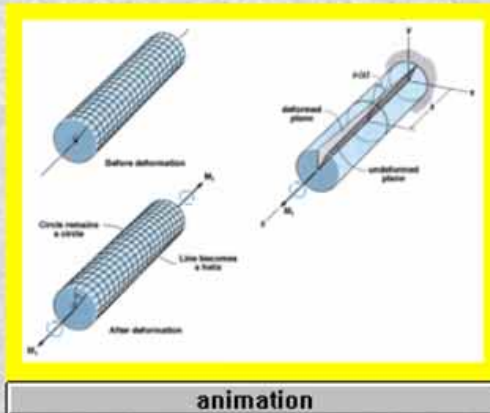
$$\begin{aligned}\sigma &= E \varepsilon \\ &= E y \kappa \\ M_x &= E I_x \kappa = -E I_x \frac{d^2 v}{dz^2} \\ \sigma &= \frac{M_x}{I_x} y\end{aligned}$$

Elementary States of Stress and Strain

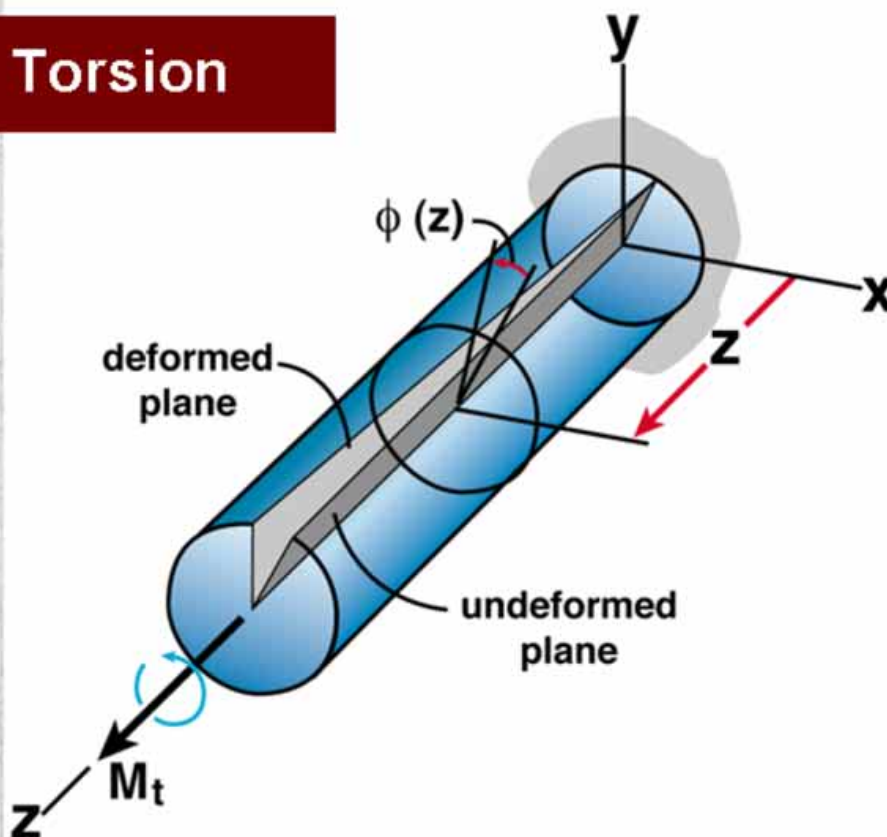
3. Torsion of Bars with Circular Cross Section

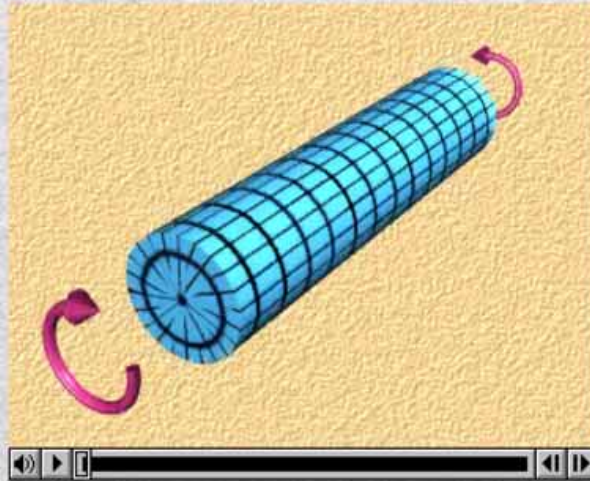
Geometry of Deformation

- Plane parallel cross sections remain plane and parallel after deformation.



Torsion





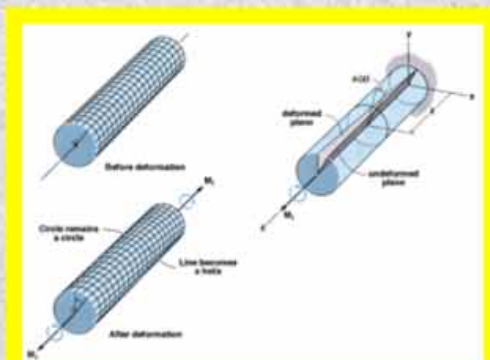
>>

Elementary States of Stress and Strain

3. Torsion of Bars with Circular Cross Section

Geometry of Deformation

- Diameters of cross sections and distances between them do not change.



animation

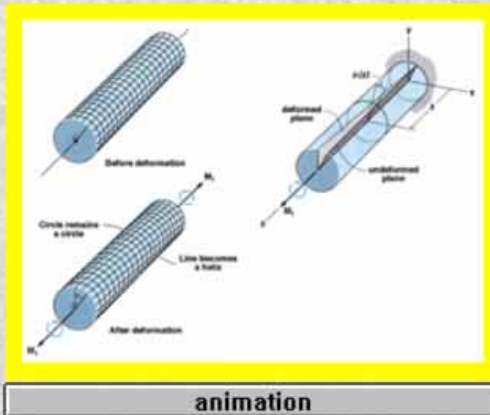
Elementary States of Stress and Strain

3. Torsion of Bars with Circular Cross Section

Geometry of Deformation

Shearing strain

$$\gamma = r \frac{d\phi}{dz}$$



Elementary States of Stress and Strain

3. Torsion of Bars with Circular Cross Section

Static Relation - Equilibrium

$$\int_A \tau r dA = M_t$$

Elementary States of Stress and Strain

3. Torsion of Bars with Circular Cross Section

Constitutive Relation

$$\begin{aligned}\tau &= G \gamma \\ M_t &= G I_p \frac{d\varphi}{dz}, \quad \left(I_p = \int_A r^2 dA \right) \\ \tau &= \frac{M_t}{I_p} r\end{aligned}$$

Elementary States of Stress and Strain

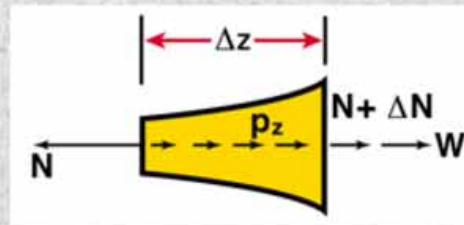
4. Relations between External and Internal Forces

Force Equilibrium

Axial Forces

$$p_z \Delta z + \Delta N = 0$$

$$\frac{dN}{dz} = -p_z$$



Elementary States of Stress and Strain

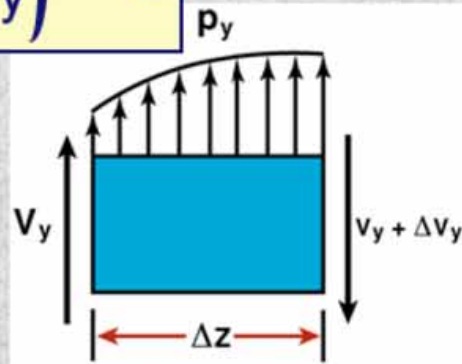
4. Relations between External and Internal Forces

Force Equilibrium

Transverse Forces

$$p_y \Delta z + V_y - (V_y + \Delta V_y) = 0$$

$$\frac{dV_y}{dz} = p_y$$

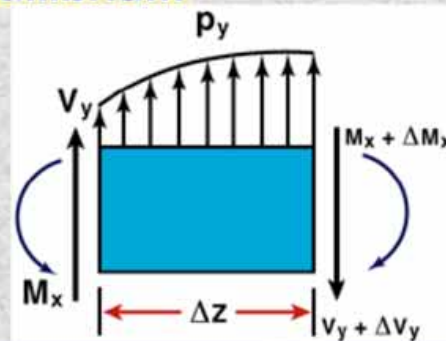


Elementary States of Stress and Strain

4. Relations between External and Internal Forces

Moment Equilibrium

Bending Moments



$$V_y \Delta z + (M_x + \Delta M_x) - M_x - p_y \frac{(\Delta z)^2}{2} = 0$$

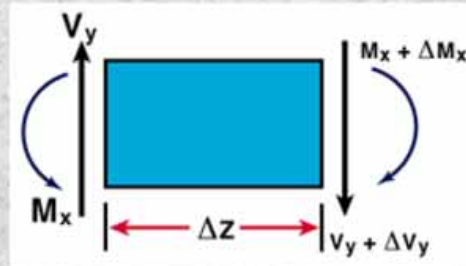
Elementary States of Stress and Strain

4. Relations between External and Internal Forces

Moment Equilibrium

Bending Moments

$$\frac{dM_x}{dz} = -V_y$$



Elementary States of Stress and Strain

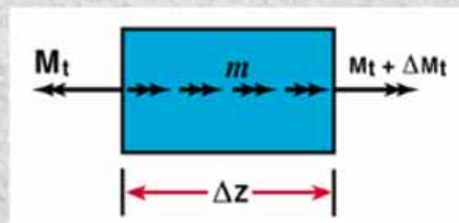
4. Relations between External and Internal Forces

Moment Equilibrium

Twisting Moments

$$\Delta z \, m + (M_t + \Delta M_t) - M_t = 0$$

$$\frac{dM_x}{dz} = -m$$



Elementary States of Stress and Strain

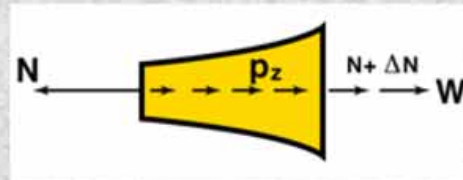
5. Governing Equations

Axial Loading

$$\frac{dN}{dz} = -p_z$$

$$N = EA \frac{dw}{dz}$$

$$\frac{d}{dz} \left(EA \frac{dw}{dz} \right) = -p_z$$



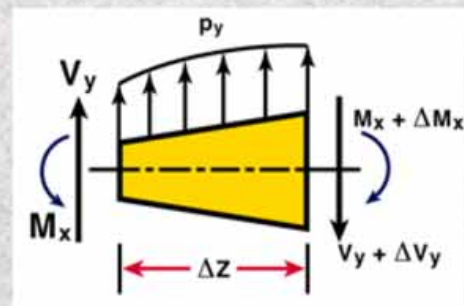
Elementary States of Stress and Strain

5. Governing Equations

Pure and Transverse Bending

$$\frac{dV_y}{dz} = p_y \quad +$$

$$\frac{dM_x}{dz} = -V_y \quad +$$

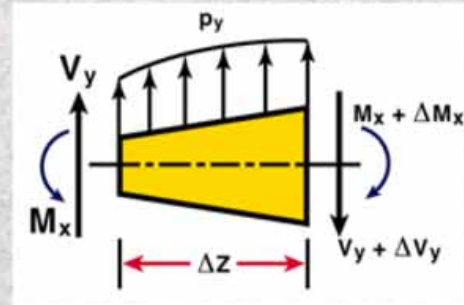


Elementary States of Stress and Strain

5. Governing Equations

+ Pure and Transverse Bending

$$\frac{d^2 M_x}{dz^2} = -p_y$$

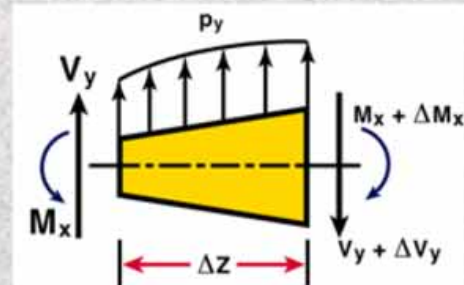


Elementary States of Stress and Strain

5. Governing Equations

+ Pure and Transverse Bending

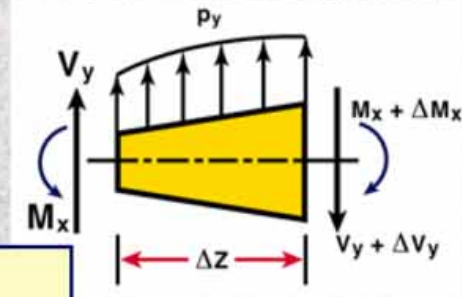
$$M_x = -E I_x \frac{d^2 v}{dz^2}$$



Elementary States of Stress and Strain

5. Governing Equations

Pure and Transverse Bending



$$\frac{d^2}{dz^2} \left(E I_x \frac{d^2 v}{dz^2} \right) = p_y$$

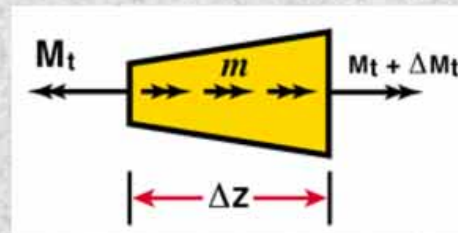
Elementary States of Stress and Strain

5. Governing Equations

Torsion

$$\frac{dM_t}{dz} = -m$$

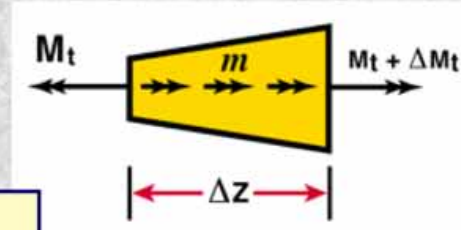
$$M_t = G I_p \frac{d\phi}{dz}$$



Elementary States of Stress and Strain



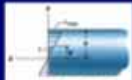
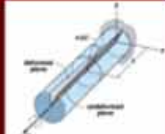
5. Governing Equations

Torsion

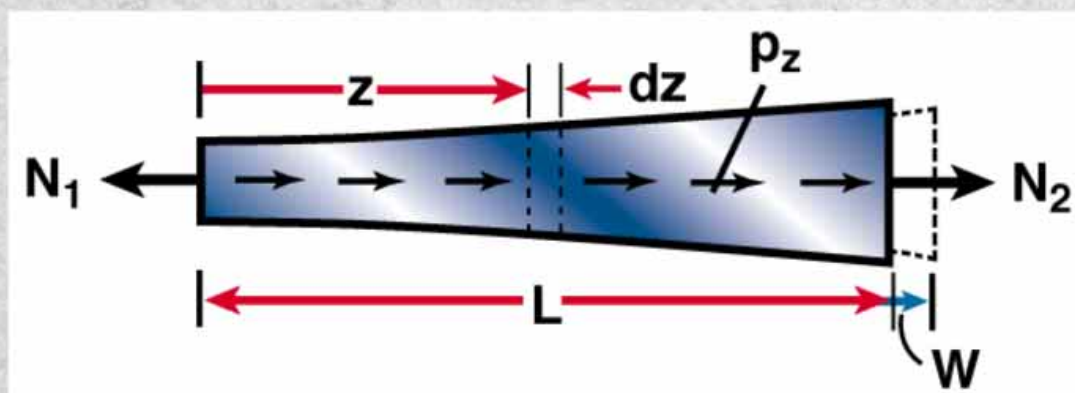


$$\frac{d}{dz} \left(G I_p \frac{d\phi}{dz} \right) = -m$$

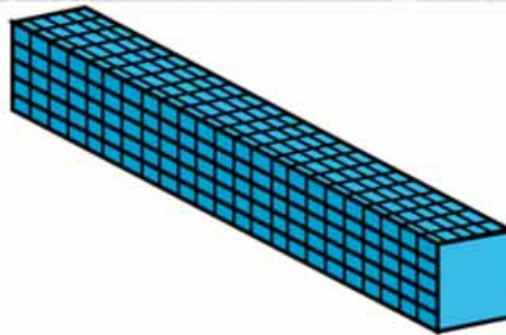
**Elementary States of
Stress and Strain
(Summary)**

		Axial Loading	Pure and Transverse Bending	Torsion
Example			 	
Kinematic Relations	Basic Assumption	Plane cross section before deformation remains plane after deformation		
	Strain Displacement Relations	$\epsilon = \frac{dw}{dz}$	$\kappa \approx -\frac{d^2v}{dz^2}$ $\epsilon = -y \frac{d^2v}{dz^2}$	$\gamma = r \frac{d\phi}{dz}$
Static Relations		$\frac{dN}{dz} = -p_z$	$\frac{dM_x}{dz} = V_y$ $\frac{dV_y}{dz} = -p_y$	$\frac{dM_t}{dz} = -m$
Constitutive Relations		$\sigma = E \epsilon$ $N = EA \epsilon$	$\sigma = E \epsilon = \frac{M_x y}{I_x}$ $M_x = E I_x \kappa$	$\tau = G \gamma = \frac{M_t r}{I_p}$ $M_t = G I_p \frac{d\phi}{dz}$

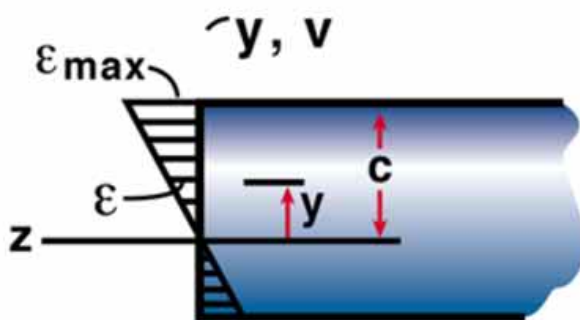
Axial Loading



Pure and Transverse Bending

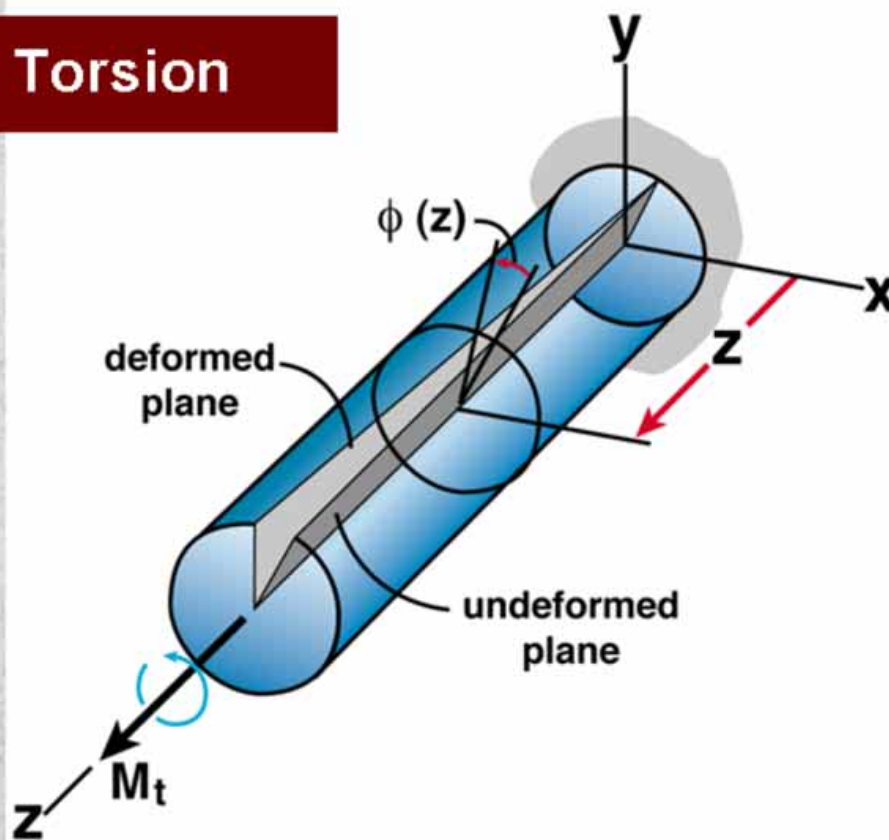


undeformed



Normal strain variation
(profile view)

Torsion



Axial Loading

Strain
Displacement
Relations

$$\varepsilon = \frac{dw}{dz}$$

Pure and Transverse Bending

Strain
Displacement
Relations

$$\kappa \cong - \frac{d^2v}{dz^2}$$

$$\varepsilon = -y \frac{d^2v}{dz^2}$$

Torsion

Strain
Displacement
Relations

$$\gamma = r \frac{d\phi}{dz}$$

Axial Loading

Static
Relations

$$\frac{dN}{dz} = -p_z$$

Pure and Transverse Bending

Static
Relations

$$\frac{dM_x}{dz} = V_y$$
$$\frac{dV_y}{dz} = -p_y$$

Torsion

Static
Relations

$$\frac{dM_t}{dz} = -m$$

Axial Loading

Constitutive
Relations

$$\begin{aligned}\sigma &= E \varepsilon \\ N &= EA \varepsilon\end{aligned}$$

Pure and Transverse Bending

Constitutive
Relations

$$\sigma = E \varepsilon = \frac{M_x y}{I_x}$$

$$M_x = E I_x \kappa$$

Torsion

Constitutive
Relations

$$\tau = G \gamma = \frac{M_t r}{I_p}$$

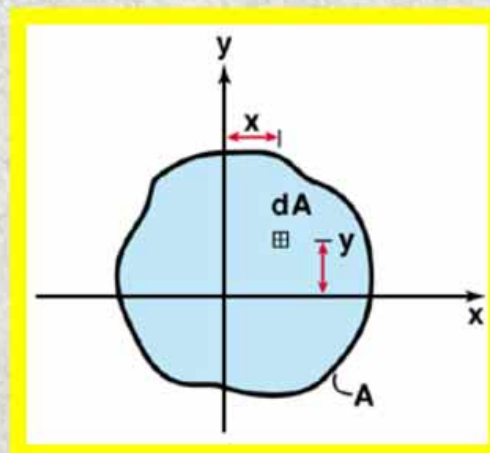
$$M_t = G I_p \frac{d\phi}{dz}$$

Geometric Properties of Plane Cross Sections

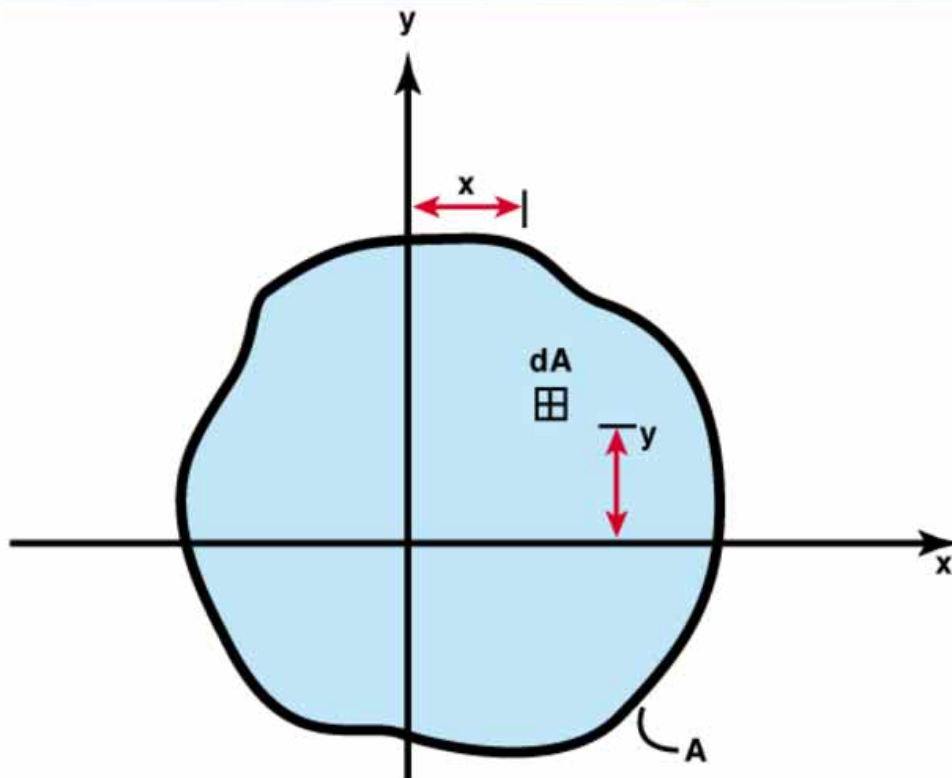
First and Second Moments:

$$A = \int_A dA$$

= area

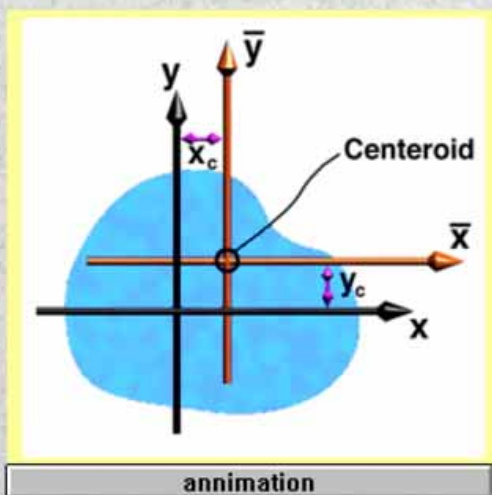


Geometric Properties of Plane Cross Sections



Geometric Properties of Plane Cross Sections

First Moments:



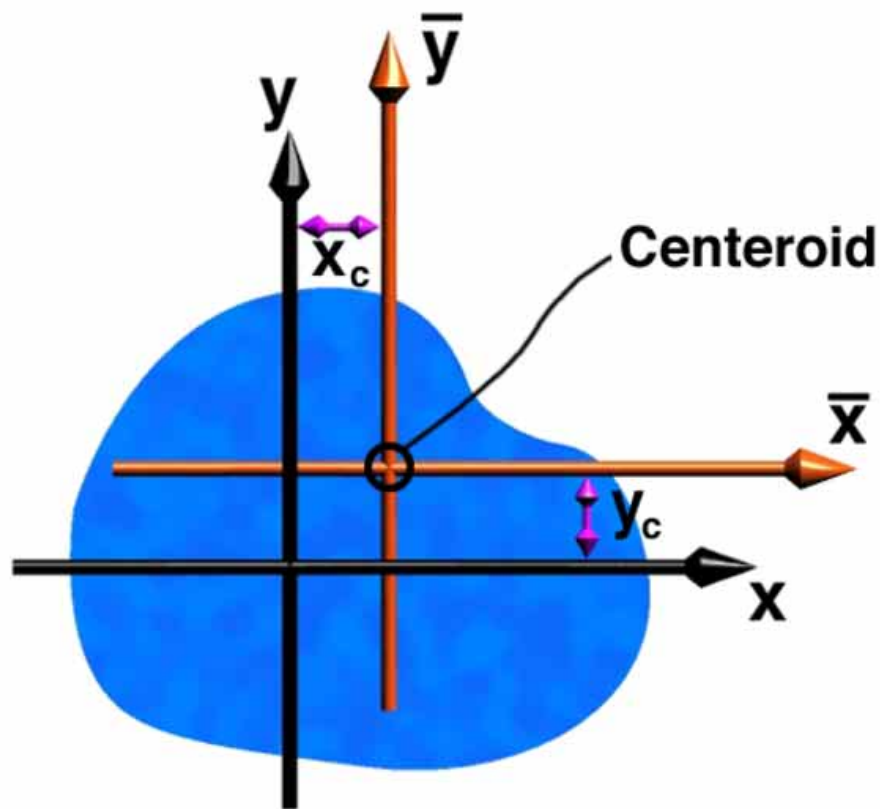
$$\begin{Bmatrix} S_x \\ S_y \end{Bmatrix} = \int_A \begin{Bmatrix} y \\ x \end{Bmatrix} dA$$

Coordinates of Centroid

$$\begin{Bmatrix} y_c \\ x_c \end{Bmatrix} = \frac{1}{A} \begin{Bmatrix} S_x \\ S_y \end{Bmatrix}$$

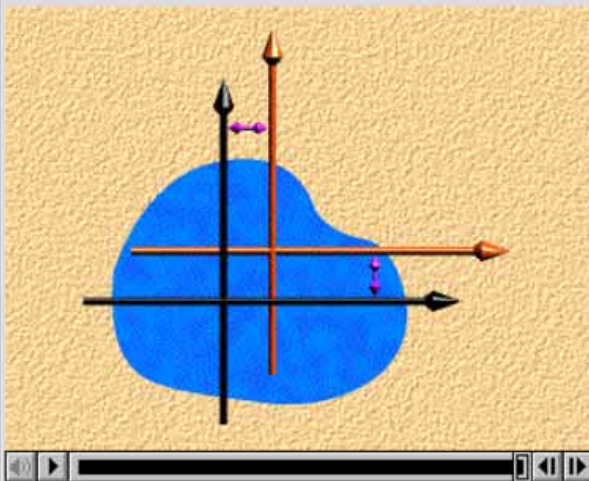
For Centroidal Axes \bar{x} , \bar{y}

$$S_x = S_y = 0$$



Geometric Properties of Plane Cross Sections

First Moments:



$$\begin{Bmatrix} S_x \\ S_y \end{Bmatrix} = \int_A \begin{Bmatrix} y \\ x \end{Bmatrix} dA$$

Coordinates of Centroid

$$\begin{Bmatrix} y_c \\ x_c \end{Bmatrix} = \frac{1}{A} \begin{Bmatrix} S_x \\ S_y \end{Bmatrix}$$

For Centroidal Axes \bar{x} , \bar{y}

$$S_x = S_y = 0$$

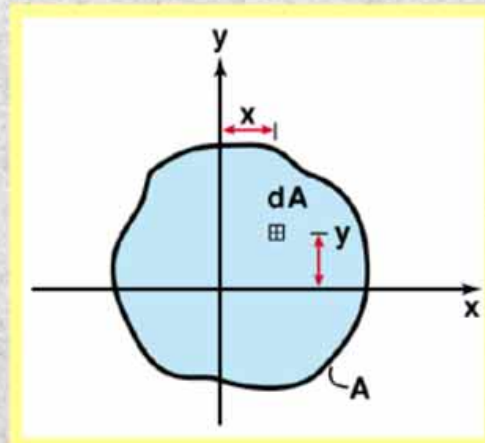
Geometric Properties of Plane Cross Sections

**Second Moments
(moments and product of Inertia):**

$$\begin{Bmatrix} I_x \\ I_y \\ I_{xy} \end{Bmatrix} = \int_A \begin{Bmatrix} y^2 \\ x^2 \\ xy \end{Bmatrix} dA$$

I_p = polar second moment
(moment of inertia)

$$= I_x + I_y$$

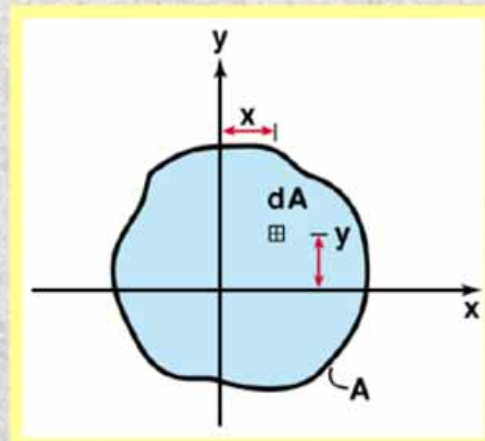


Geometric Properties of Plane Cross Sections

Radii of Gyration:

$$\begin{Bmatrix} r_x^2 \\ r_y^2 \end{Bmatrix} = \frac{1}{A} \begin{Bmatrix} I_x \\ I_y \end{Bmatrix}$$

r_x, r_y are radii of gyration with respect to x and y axes



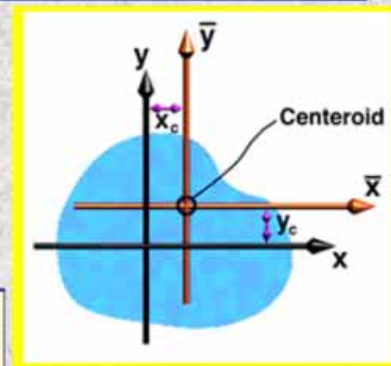
Geometric Properties of Plane Cross Sections

Effect of Translation of Coordinates

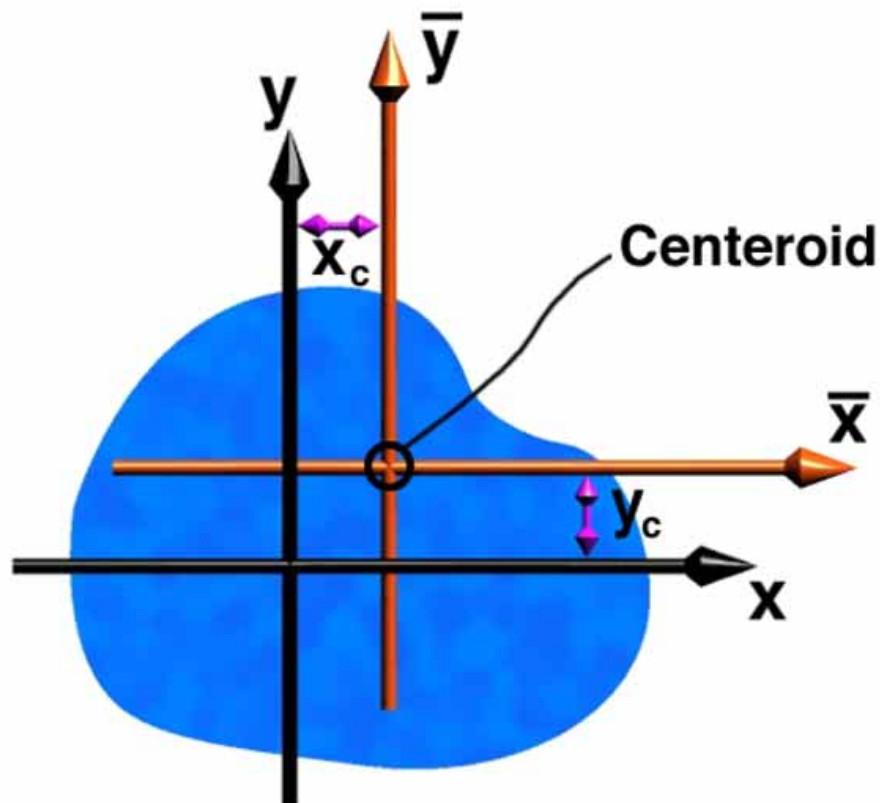
Translation of Coordinates

\bar{x} , \bar{y} are centroidal coordinates

$$\begin{Bmatrix} I_x \\ I_y \\ I_{xy} \end{Bmatrix} = \begin{Bmatrix} I_{\bar{x}} \\ I_{\bar{y}} \\ I_{\bar{x}\bar{y}} \end{Bmatrix} + A \begin{Bmatrix} y_c^2 \\ x_c^2 \\ x_c y_c \end{Bmatrix}$$



animation

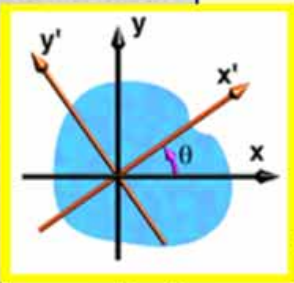


Geometric Properties of Plane Cross Sections

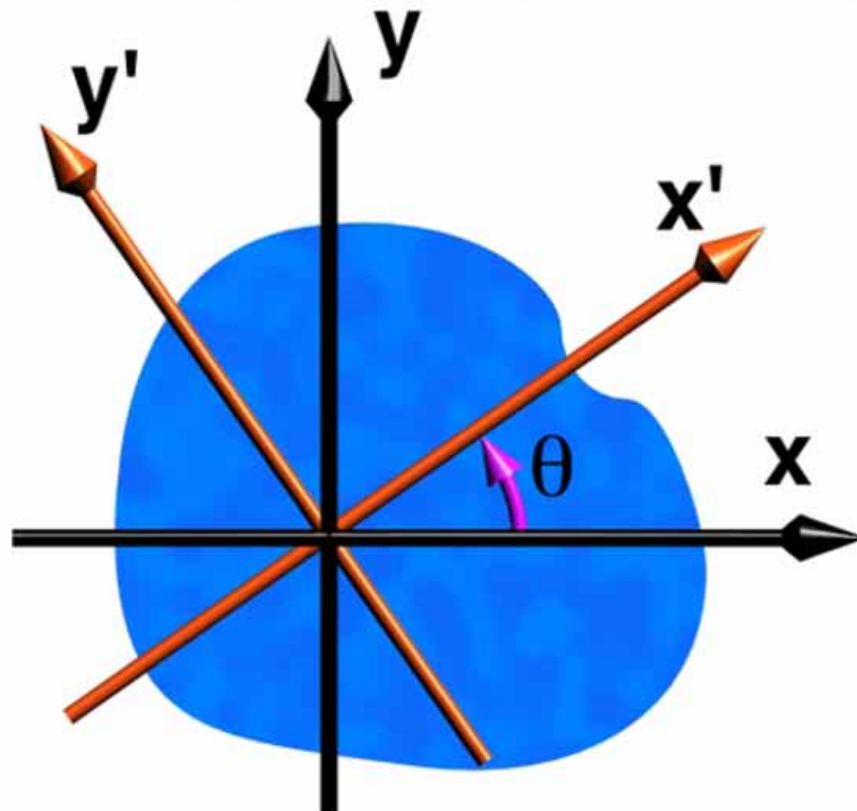
Rotation of Coordinates:

$$\begin{pmatrix} I_{x'} \\ I_{y'} \\ I_{x'y'} \end{pmatrix} = \begin{vmatrix} \cos^2\theta & \sin^2\theta & -2\sin\theta\cos\theta \\ \sin^2\theta & \cos^2\theta & 2\sin\theta\cos\theta \\ \sin\theta\cos\theta & -\sin\theta\cos\theta & \cos^2\theta - \sin^2\theta \end{vmatrix} \begin{pmatrix} I_x \\ I_y \\ I_{xy} \end{pmatrix}$$

$$= \begin{vmatrix} \frac{1}{2}(1 + \cos 2\theta) & \frac{1}{2}(1 - \cos 2\theta) & -\sin 2\theta \\ \frac{1}{2}(1 - \cos 2\theta) & \frac{1}{2}(1 + \cos 2\theta) & \sin 2\theta \\ \frac{1}{2}\sin 2\theta & -\frac{1}{2}\sin 2\theta & \cos 2\theta \end{vmatrix} \begin{pmatrix} I_x \\ I_y \\ I_{xy} \end{pmatrix}$$



animation



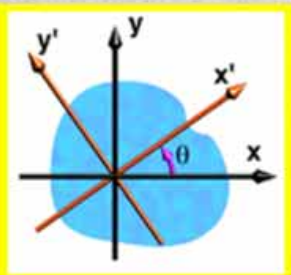
Geometric Properties of Plane Cross Sections

Principal Axes:

$I_{x'}, I_{y'}$ are maximum and minimum second moments

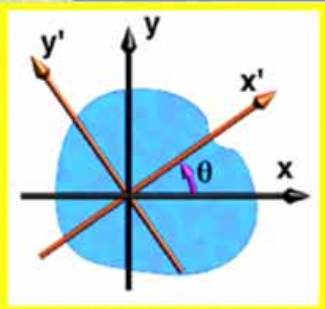
$$I_{x'y'} = 0$$

$$\tan 2\theta = \frac{-2 I_{xy}}{I_x - I_y}$$



animation

Mohr's Circle Representation of Moments and Product of Inertia

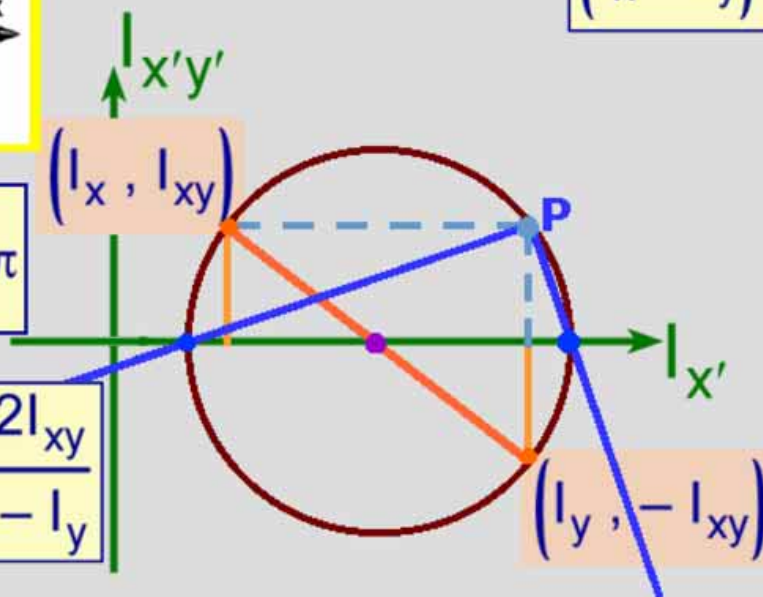


$$I_{xy} > 0$$

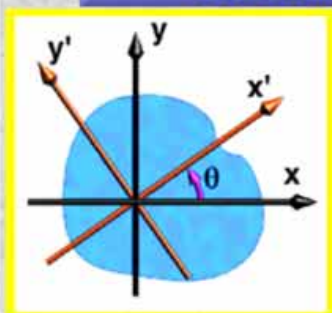
$$(I_x - I_y) < 0$$

$$\pi < 2\theta < \frac{3}{2}\pi$$

$$\tan 2\theta = \frac{-2 I_{xy}}{I_x - I_y}$$



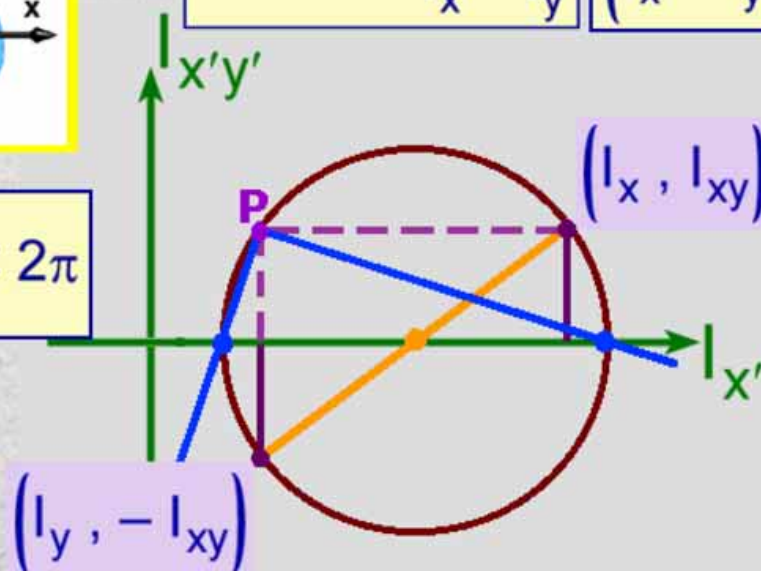
Mohr's Circle Representation of Moments and Product of Inertia



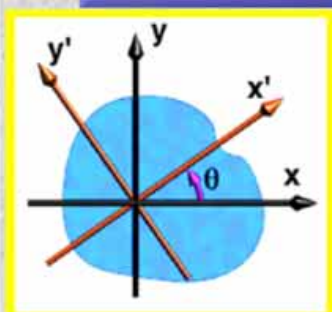
$$\tan 2\theta = \frac{-2I_{xy}}{I_x - I_y}$$

$$\begin{matrix} I_{xy} > 0 \\ (I_x - I_y) > 0 \end{matrix}$$

$$\frac{3}{2}\pi < 2\theta < 2\pi$$



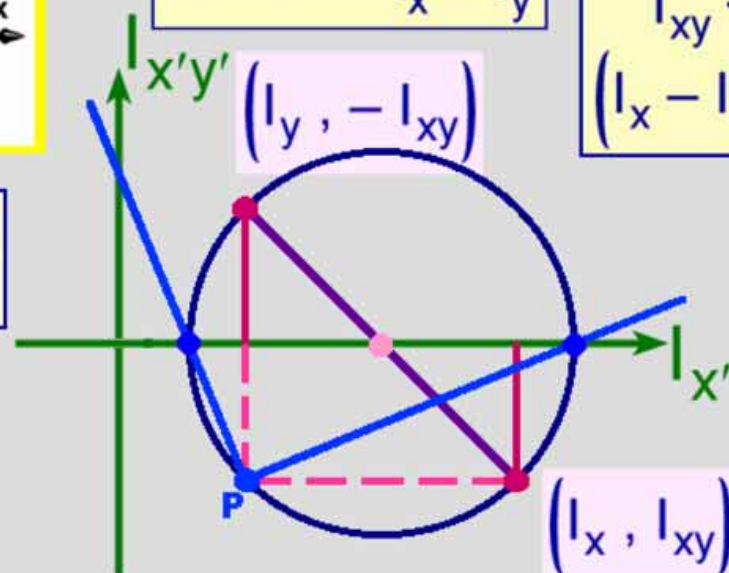
Mohr's Circle Representation of Moments and Product of Inertia



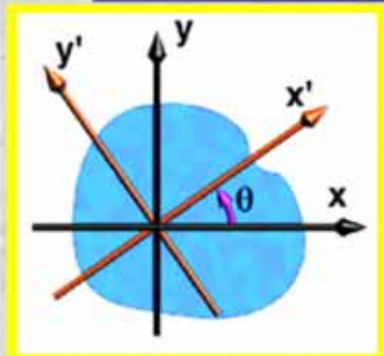
$$\tan 2\theta = \frac{-2I_{xy}}{I_x - I_y}$$

$$\begin{matrix} I_{xy} < 0 \\ (I_x - I_y) > 0 \end{matrix}$$

$$0 < 2\theta < \frac{\pi}{2}$$



Mohr's Circle Representation of Moments and Product of Inertia



$$\frac{\pi}{2} < 2\theta < \pi$$

$$\tan 2\theta = \frac{-2I_{xy}}{I_x - I_y}$$

$$I_{xy} < 0$$

$$(I_x - I_y) < 0$$

